City of Bowman
Bowman City Hall Roof Replacement
Bowman, North Dakota

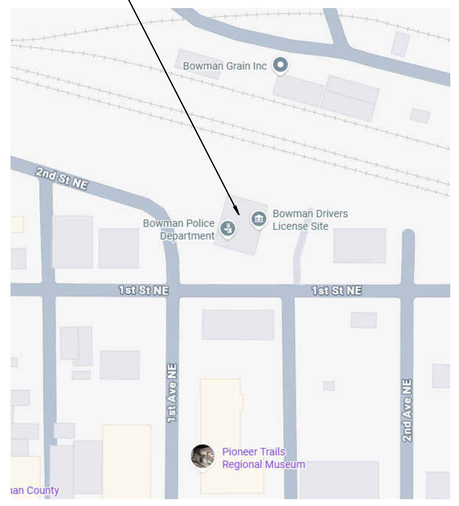
Contract Documents

February 11, 2026

INDEX

VICINITY MAP

CITY OF BOWMAN
101 1ST ST NE
BOWMAN, ND 58623



SET #

City of Bowman Bowman City Hall Roof Replacement

February 11, 2026

Contract Documents

J2 Studio Project #: J22528

STRUCTURAL

CWSTRUCTURAL ENGINEERS

CWSTRUCTURAL

100 E. CALGARY AVENUE, SUITE 2
BISMARCK, ND 58503
CONTACT: CHRIS WENTZ, PE
PHONE: (701) 221-3286

ARCHITECT

J2 studio architecture + design, pc

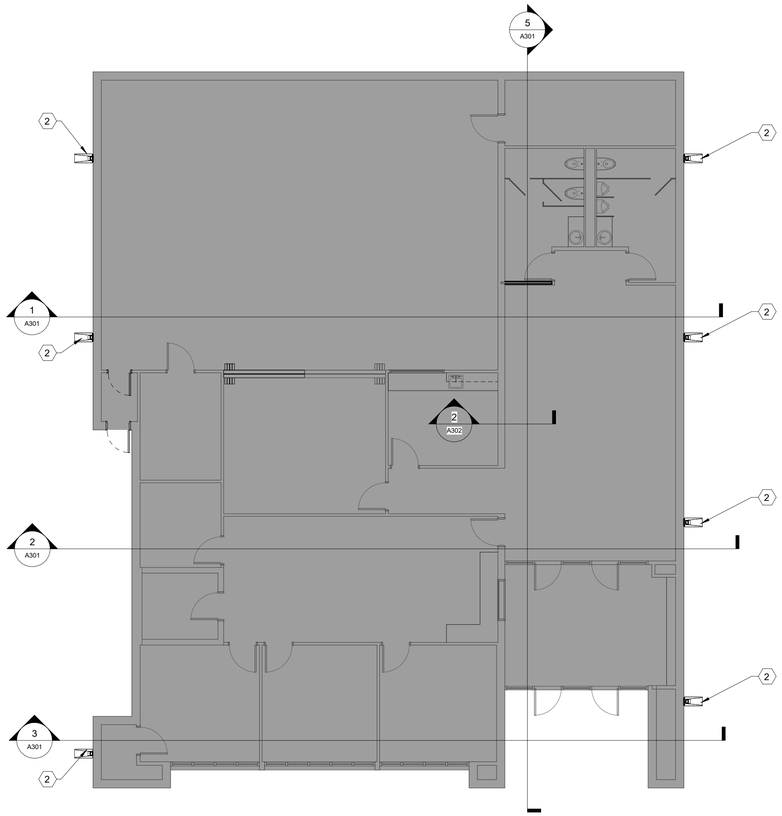
J2 STUDIO

919 SOUTH 7TH STREET, SUITE 400
BISMARCK, ND 58504
CONTACT: JAMES DEVINE, AIA, LEED AP
PHONE: (701) 255-1622

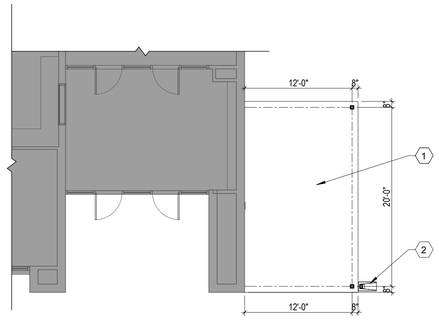
OWNER

CITY OF BOWMAN

101 1ST ST NE
BOWMAN, ND 58623
CONTACT: PEGGY ALLEN, FINANCIAL AUDITOR
PHONE: (701) 523-3309



1 Main Floor Renovation Plan
Scale: 1/8" = 1'-0"



2 Partial Main Floor Renovation Plan (Alt. Bid)
Scale: 1/8" = 1'-0"

NOTE:

INTERIOR FLOOR PLAN SHOWN FOR REFERENCE ONLY

LEGEND:

- OFFICE / 101 — ROOM NAMES: ROOM NAME, ROOM NUMBER
- 1 / A201 — BUILDING ELEVATIONS / SECTIONS: ELEVATION NUMBER, SHEET NUMBER
- 101 / A — DOORS: DOOR NUMBER, DOOR LETTER, REFER TO SHEET A601 FOR SCHEDULE
- 1 — WINDOWS: WINDOW TYPE, REFER TO SHEET A601 FOR SCHEDULE
- A / 1 — WALL TYPES: WALL TYPE, WALL TYPE NOTE
- 1 / A601 — DETAIL MARK: DETAIL NUMBER, SHEET NUMBER
- 1 / A701 — INTERIOR ELEVATION: ELEVATION NUMBER, SHEET NUMBER
- T1 — EQUIPMENT: EQUIPMENT NUMBER
- 1 — DEMOLITION KEYNOTE: ITEM NUMBER - REFER TO SPECIFIC DEMOLITION NOTES
- 1 — PLAN KEYNOTE: ITEM NUMBER - REFER TO SPECIFIC NOTES

GENERAL PLAN NOTES:

1. GENERAL CONTRACTOR TO COORDINATE CONSTRUCTION ACTIVITIES WITH OWNER.
2. GENERAL CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS PRIOR TO COMMENCEMENT OF WORK.
3. GENERAL CONTRACTOR TO COORDINATE OWNER PROVIDED EQUIPMENT INSTALLATION.
4. ALL WORK SHALL MEET ALL APPLICABLE BUILDING CODES AND REQUIREMENTS.

SPECIFIC PLAN NOTES:

1. CONCRETE SLAB; REFER TO STRUCTURAL
2. SPLASHBLOCK; REFER TO DETAIL 1/A103

REVISIONS

MARK	DATE	DESCRIPTION

Contract Documents

February 11, 2026

J2 Project No. J22528

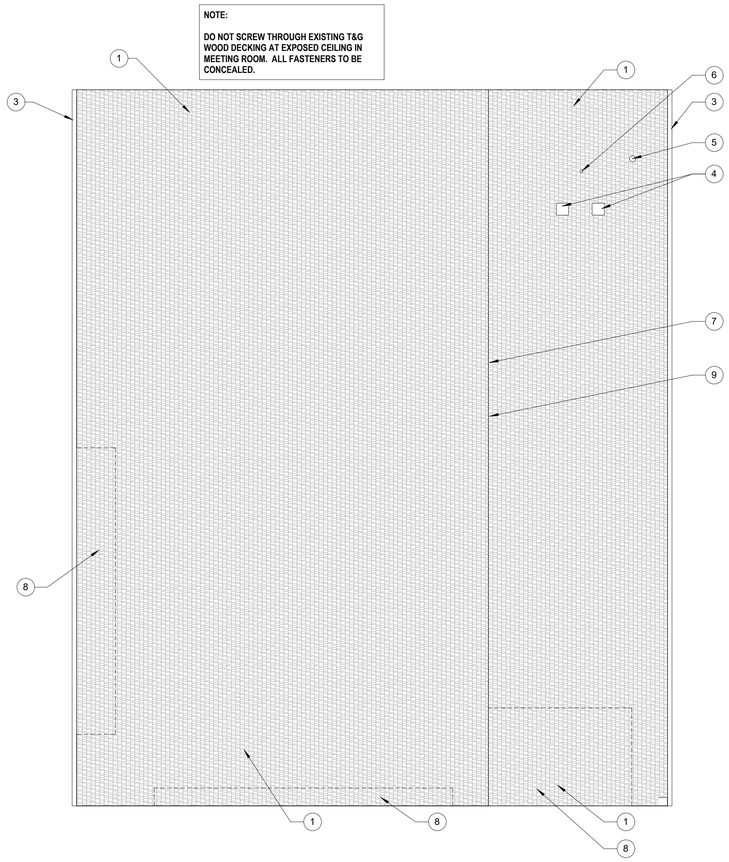
LICENSED ARCHITECT
JAMES DEVINE
(1292)
DATE: 02/11/2026
STATE OF NORTH DAKOTA

City of Bowman
Bowman City Hall Roof Replacement
Bowman, North Dakota

J2 studio
architecture + design, pc
919 South 7th Street, Suite 400
Bismarck, ND 58504
(701) 255-1622

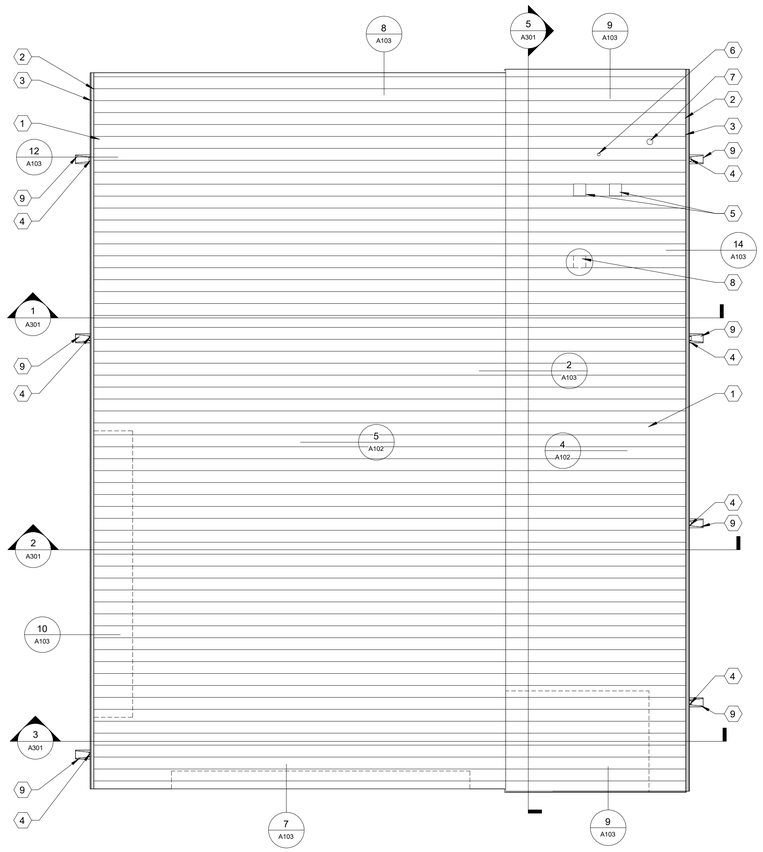
Main Floor & Partial Main Floor Plans

A101

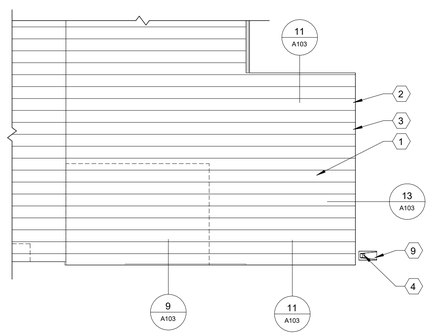


1 Roof Demolition Plan
Scale: 1/8" = 1'-0"

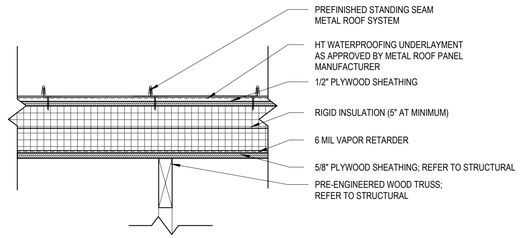
NOTE:
EXISTING DRAWINGS INDICATE A CONCEALED GUTTER. G.C. TO REMOVE CONCEALED GUTTER, IF EXISTS AND ALL ADDITIONAL BLOCKING/NAILERS AND FRAMING FOR CONCEALED SYSTEM AND REPLACE WITH NEW NAILERS. EXISTING SHINGLES, ROOF INSULATION AND VAPOR RETARDER AND/OR BUILDING FELT SHALL ALSO BE REMOVED PRIOR TO RE-ROOFING.



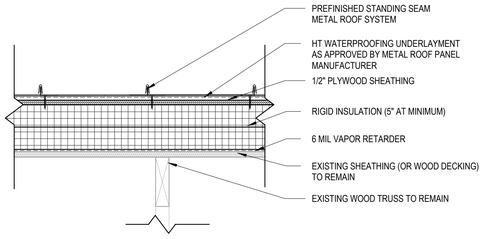
2 Roof Plan
Scale: 1/8" = 1'-0"



3 Partial Roof Plan (Alternate Bid)
Scale: 1/8" = 1'-0"



4 Typical Roof Section at Standing Seam Roof
Scale: 1 1/2" = 1'-0"



5 Typical Existing Roof Section at Standing Seam Roof
Scale: 1 1/2" = 1'-0"

GENERAL ROOF PLAN NOTES:

1. THE FOLLOWING REFERENCES HAVE BEEN USED AS A BASIS OF DESIGN FOR THE ROOFING WORK OF THE PROJECT AND SHALL BE USED BY THE CONTRACTOR TO DETERMINE REQUIREMENTS FOR FABRICATION AND/OR INSTALLATION WHEN NOT SPECIFICALLY INDICATED IN THE CONTRACT DOCUMENTS:
   - 1.1 2021 INTERNATIONAL BUILDING CODE
   - 1.2 MATERIAL MANUFACTURER'S MOST RECENT PRINTED SPECIFICATIONS AND DETAILS.
   - 1.3 ALL OTHER APPLICABLE CODES AND REGULATIONS FOR THE CITY OF BOWMAN.
2. PRIOR TO THE START OF WORK, THE CONTRACTOR SHALL PROVIDE AND INSTALL PROTECTION OVER, UNDER, AND/OR AROUND ALL SERVICE LINES, BUILDING COMPONENTS, SIDEWALKS, PAVEMENT, AND LANDSCAPING WHICH COULD BE DAMAGED OR SOILED WHILE PERFORMING THE WORK OF THE CONTRACT.
3. DETAILS IN THE PROJECT DRAWINGS ARE SHOWN AT SPECIFIC LOCATIONS AND ARE INTENDED TO SHOW GENERAL REQUIREMENTS THROUGHOUT.
   - 3.1 DETAILS NOTED ARE 'TYPICAL' AND IMPLY SIMILAR CONDITIONS TREATED SIMILARLY. MODIFICATIONS TO BE MADE BY THE CONTRACTOR TO ACCOMMODATE MINOR VARIATIONS WITHOUT ADDITIONAL COST TO THE OWNER.
4. ALL PENETRATION DETAILS PER MANUFACTURER'S STANDARD DETAILS.
5. ALL SYSTEMS SHALL BE PER MANUFACTURER'S STANDARDS FOR PRODUCT WARRANTIES. CONTRACTOR TO PROVIDE COMPLETE ROOFING SYSTEM, INCLUDING, BUT NOT LIMITED TO, UNDERLAYMENTS, ICE AND WATER BARRIERS, VAPOR RETARDERS, COVER BOARDS, AND INSULATION.
6. MECHANICAL PENETRATIONS; TERMINATE AND FLASH PER ROOFING MANUFACTURER RECOMMENDATIONS.
7. PLUMBING PENETRATIONS; TERMINATE AND FLASH PER ROOFING MANUFACTURER RECOMMENDATIONS.

SPECIFIC DEMOLITION NOTES:

1. REMOVE EXISTING ASPHALT SHINGLES, NAIL BASE, INSULATION IN ITS ENTIRETY TO EXPOSE EXISTING WOOD DECK/WOOD SHEATHING.
2. REMOVE EXISTING NAILERS AS REQUIRED FOR NEW FRAMING/ROOFING.
3. REMOVE EXISTING GUTTER AND DOWNSPOUT.
4. REMOVE EXISTING VENT CAP. EXTEND VENTING TO NEW ROOF AND INSTALL NEW VENT CAP.
5. REMOVE EXISTING FLUE AND STORE FOR REINSTALLATION. EXTEND FLUE TO NEW ROOF.
6. REMOVE EXISTING FLASHING AT VENT THROUGH ROOF. EXTEND VENT TO NEW ROOF.
7. REMOVE EXISTING EXHAUST LOUVER ON FACE OF WALL. EXTEND TO NEW ROOF AND PROVIDE NEW ROOFTOP EXHAUST LOUVER.
8. REMOVE EXISTING SOFFIT LIGHTS. ELECTRICAL BOXES AND WIRING TO REMAIN. REPLACE WITH NEW LIGHTS. REFER TO ALLOWANCES IN SPECIFICATIONS.
9. REMOVE EXISTING FACE BRICK AT ROOF STEP; REFER TO STRUCTURAL AND ARCHITECTURAL SECTIONS

SPECIFIC ROOF PLAN NOTES:

1. PREFINISHED STANDING SEAM METAL ROOF PANEL; PAC-CLAD TITE-LOC; COLOR - DARK BRONZE
2. PREFINISHED METAL DRIP EDGE; PAC-CLAD; COLOR - DARK BRONZE
3. PREFINISHED METAL GUTTER; PAC-CLAD; COLOR - DARK BRONZE
4. PREFINISHED METAL DOWNSPOUT; PAC-CLAD; COLOR - DARK BRONZE
5. EXHAUST HOOD BY MECHANICAL CONTRACTOR. EXTEND EXISTING DUCTWORK TO NEW ROOF PENETRATION. FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
6. PLUMBING VENT BY MECHANICAL CONTRACTOR. EXTEND EXISTING VENT PIPING THROUGH ROOF AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
7. MECHANICAL FLUE BY MECHANICAL CONTRACTOR. EXTEND EXISTING VENT PIPING THROUGH ROOF AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
8. MECHANICAL EXHAUST HOOD BY MECHANICAL CONTRACTOR. EXTEND EXISTING DUCTWORK TO ROOF AND PENETRATE WITH NEW ROOF HOOD AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
9. SPLASHBLOCK; REFER TO DETAIL 1/A103

REVISIONS

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Contract Documents

February 11, 2026

J2 Project No. J22528

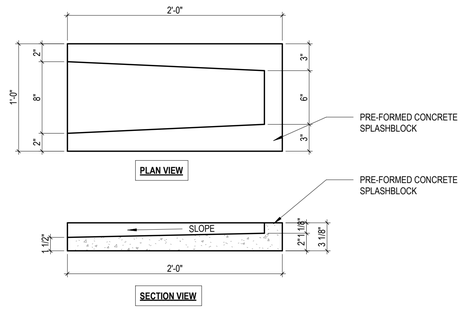
LICENSED ARCHITECT
JAMES DEVINE
(1292)
DATE: 02/11/2026
STATE OF NORTH DAKOTA

City of Bowman
Bowman City Hall Roof Replacement
Bowman, North Dakota

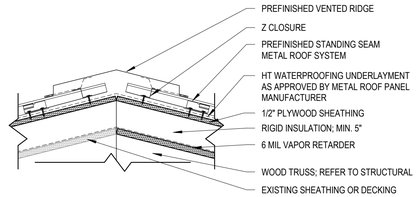
J2 studio
architecture + design, pc
919 South 7th Street, Suite 400
Bismarck, ND 58504
(701) 255-1622

Roof Demolition and Roof Plans

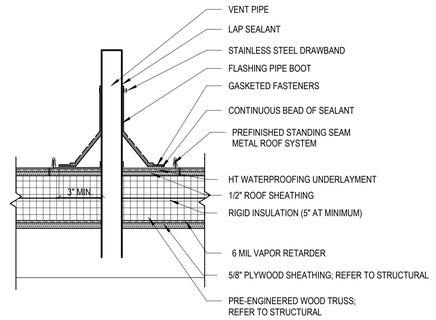
A102



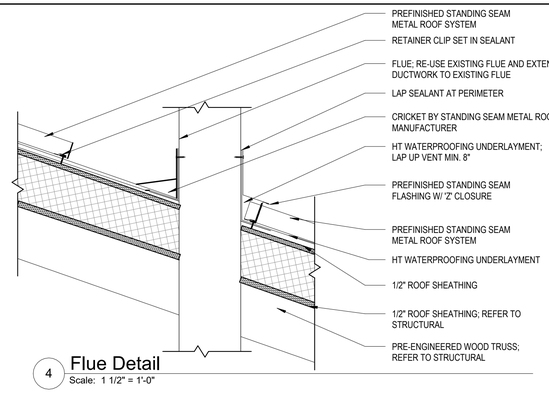
1 Splashblock Detail
Scale: 1 1/2" = 1'-0"



2 Ridge Vent Detail
Scale: 1 1/2" = 1'-0"



3 Typical Vent Thru Roof Detail
Scale: 1 1/2" = 1'-0"



4 Flue Detail
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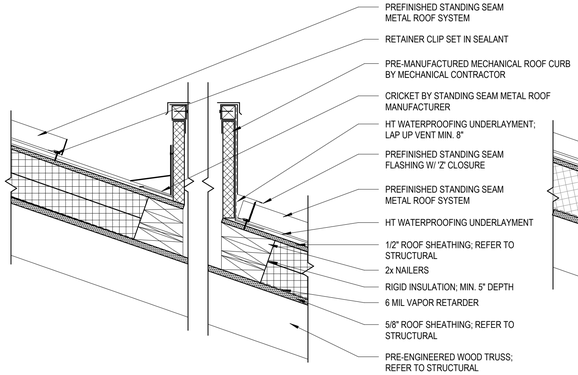
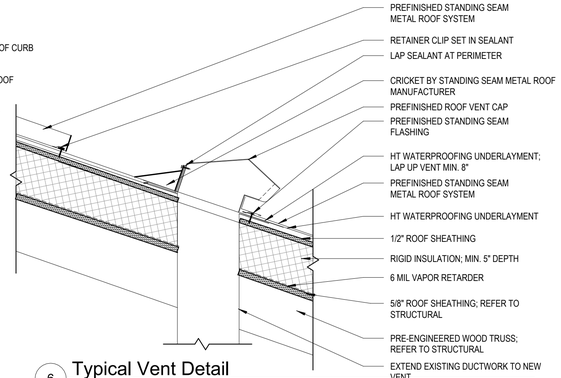
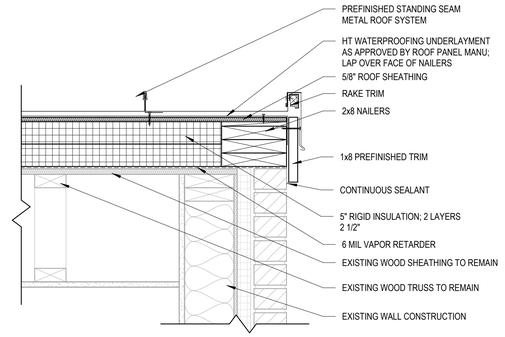


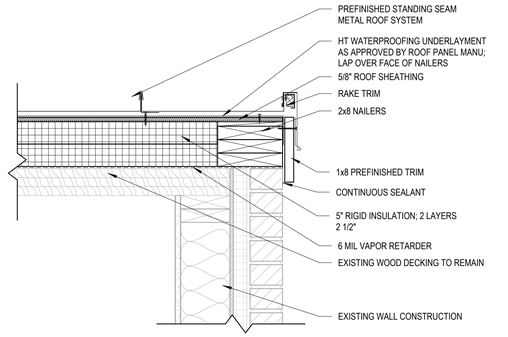
5 Typical Roof Curb Detail
Scale: 1 1/2" = 1'-0"



6 Typical Vent Detail
Scale: 1 1/2" = 1'-0"



7 Rake Detail at Existing Wall
Scale: 1 1/2" = 1'-0"



8 Rake Detail at Existing Wall
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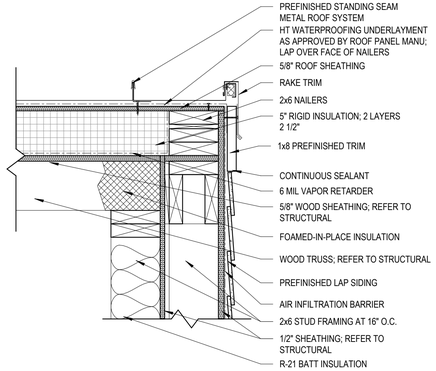
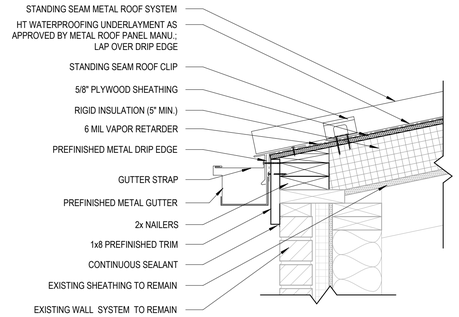
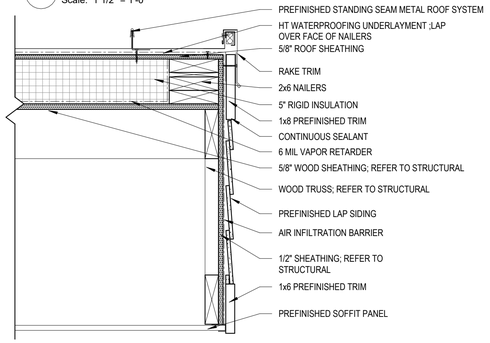


9 Rake Detail at New Wall
Scale: 1 1/2" = 1'-0"



10 Roof Detail - Standing Seam at Gutter
Scale: 1 1/2" = 1'-0"



11 Rake Detail at Patio
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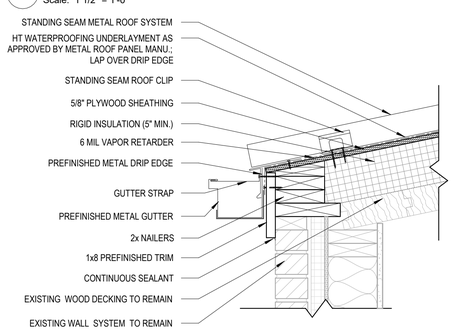
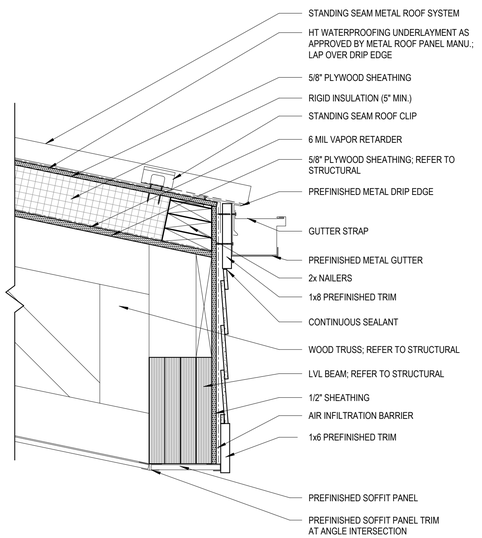
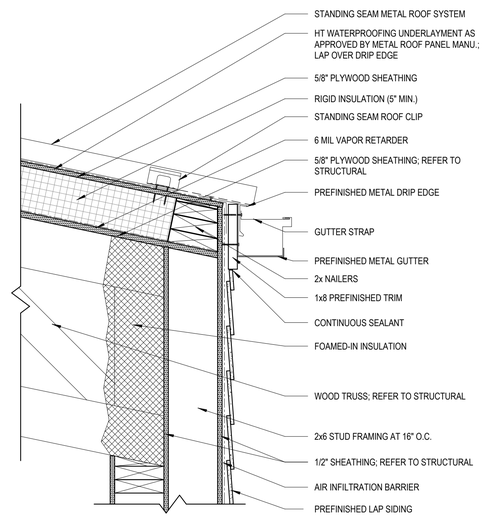


12 Roof Detail - Standing Seam at Gutter
Scale: 1 1/2" = 1'-0"



13 Roof Detail - Standing Seam at Gutter
Scale: 1 1/2" = 1'-0"



14 Roof Detail - Standing Seam at Gutter
Scale: 1 1/2" = 1'-0"

REVISIONS

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J2 Project No. J22528

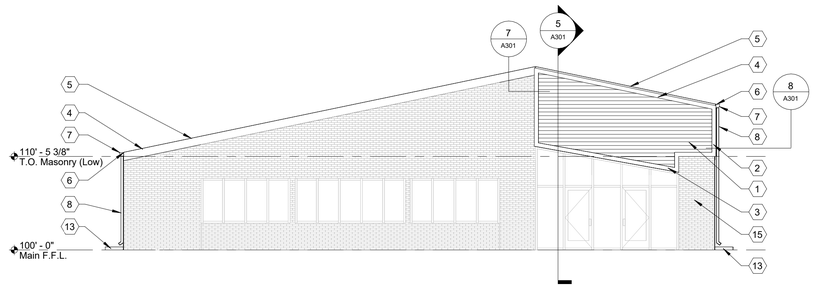
LICENSED ARCHITECT JAMES DEVINE (1292) DATE: 02/11/2026 STATE OF NORTH DAKOTA

City of Bowman Bowman City Hall Roof Replacement
Bowman, North Dakota

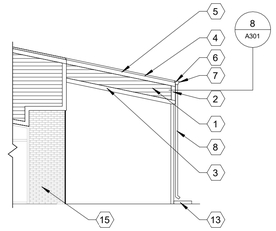
J2 studio architecture + design, pc
919 South 7th Street, Suite 400
Bismarck, ND 58504
(701) 255-1622

Roof Details

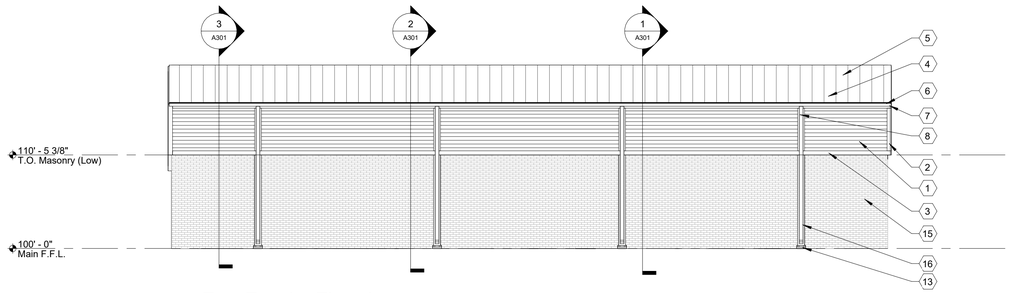
A103



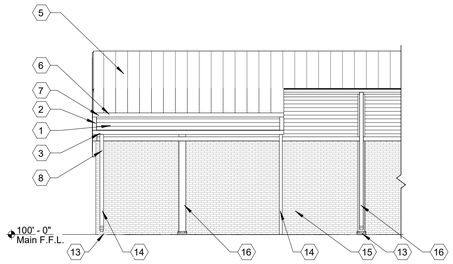
1 South Exterior Elevation
Scale: 1/8" = 1'-0"



2 Partial South Exterior Elevation (Alt. Bid)
Scale: 1/8" = 1'-0"



3 East Exterior Elevation
Scale: 1/8" = 1'-0"



4 Partial East Exterior Elevation (Alt. Bid)
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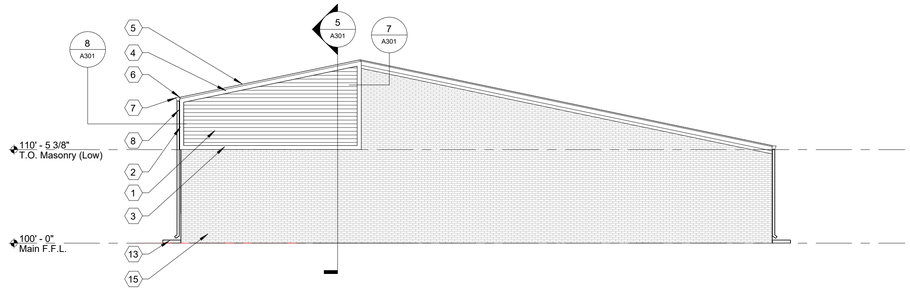
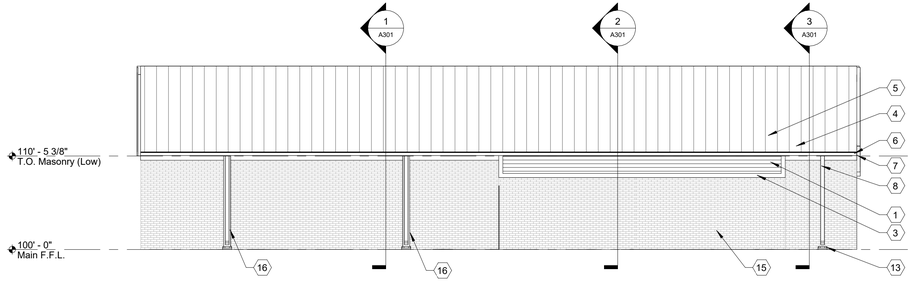
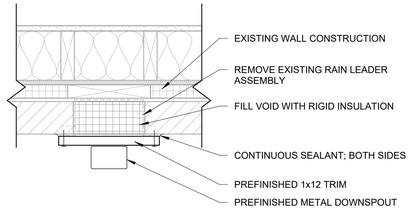


5 North Exterior Elevation
Scale: 1/8" = 1'-0"



6 West Exterior Elevation
Scale: 1/8" = 1'-0"



7 Existing Downspout Infill Detail
Scale: 1 1/2" = 1'-0"

GENERAL ELEVATION NOTES:

1. GENERAL CONTRACTOR TO COORDINATE CONSTRUCTION ACTIVITIES WITH OWNER.

SPECIFIC ELEVATION NOTES:

1. EXTERIOR SIDING; LP SMARTSIDE; COLOR - TUNDRA GRAY, SMOOTH FINISH; 5" EXPOSURE
2. PREFINISHED CORNER TRIM; LP SMARTSIDE TRIM; COLOR - MIDNIGHT SHADOW; SMOOTH FINISH
3. PREFINISHED 1x6 TRIM; LP SMARTSIDE TRIM; COLOR - MIDNIGHT SHADOW; SMOOTH FINISH
4. PREFINISHED 1x8 TRIM; LP SMARTSIDE TRIM; COLOR - MIDNIGHT SHADOW; SMOOTH FINISH
5. PREFINISHED STANDING SEAM METAL ROOF PANEL; PAC-CLAD TITE-LOC; COLOR - DARK BRONZE
6. PREFINISHED METAL DRIP EDGE; PAC-CLAD; COLOR - DARK BRONZE
7. PREFINISHED METAL GUTTER; PAC-CLAD; COLOR - DARK BRONZE
8. PREFINISHED METAL DOWNSPOUT; PAC-CLAD; COLOR - DARK BRONZE
9. EXHAUST HOOD BY MECHANICAL CONTRACTOR. EXTEND EXISTING DUCTWORK TO NEW ROOF PENETRATION. FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
10. PLUMBING VENT BY MECHANICAL CONTRACTOR. EXTEND EXISTING VENT PIPING THROUGH ROOF AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
11. MECHANICAL FLUE BY MECHANICAL CONTRACTOR. EXTEND EXISTING VENT PIPING THROUGH ROOF AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
12. MECHANICAL EXHAUST HOOD BY MECHANICAL CONTRACTOR. EXTEND EXISTING DUCTWORK TO ROOF AND PENETRATE WITH NEW ROOF HOOD AND FLASH PER ROOF MANUFACTURER'S RECOMMENDATIONS.
13. SPLASHBLOCK; REFER TO DETAIL 1/A103
14. STEEL COLUMN; PAINT TO MATCH SMARTSIDE TRIM COLOR
15. EXISTING BUILDING
16. PREFINISHED 1x12 TRIM; LP SMARTSIDE TRIM; COLOR - MIDNIGHT SHADOW; SMOOTH FINISH; REFER TO DETAIL 7/A201

NOTE; FINAL COLOR SELECTIONS FOR SIDING, SIDING TRIM, SOFFIT PANELS, PREFINISHED METAL ROOFING ACCESSORIES, PREFINISHED FASCIA AND PREFINISHED STANDING SEAM ROOF TO BE SELECTED FROM MANUFACTURER'S FULL RANGE OF STANDARD COLORS.

REVISIONS

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LICENSED ARCHITECT
JAMES DEVINE
(1292)
DATE: 02/11/2026
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City of Bowman
Bowman City Hall Roof Replacement
Bowman, North Dakota

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architecture + design, pc
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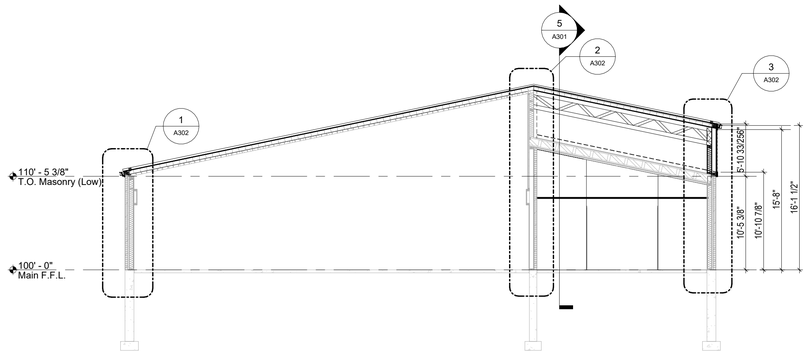
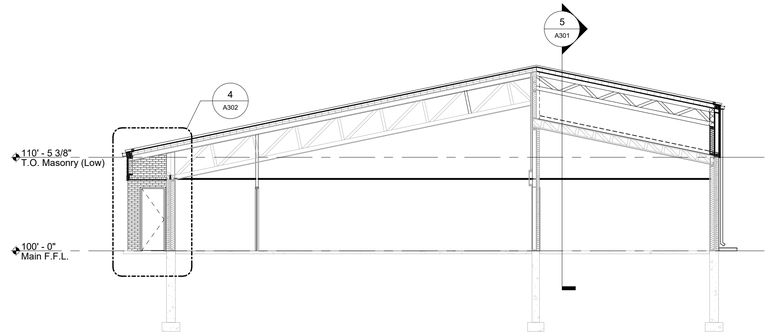
Exterior Elevations & Axon Views

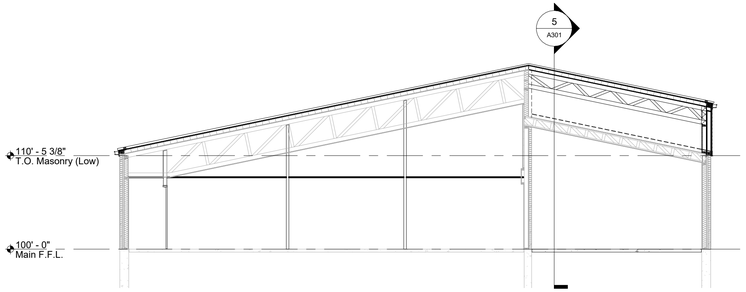
A201



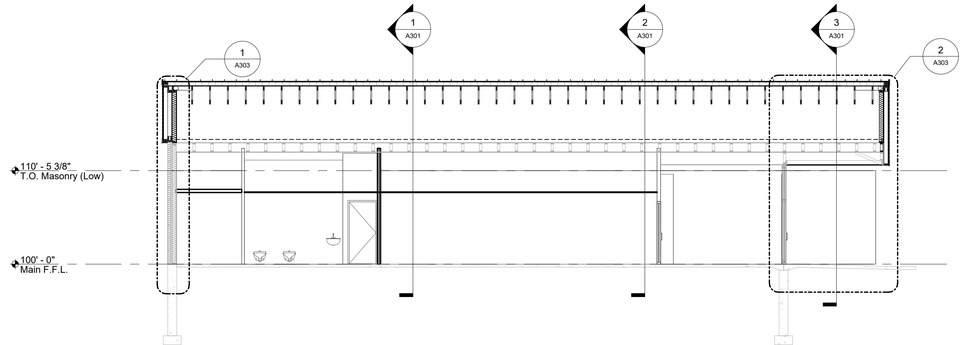
1 Building Section
Scale: 1/8" = 1'-0"



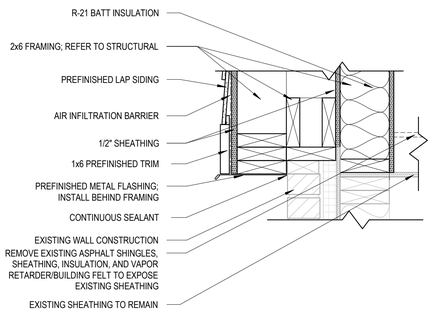
2 Building Section
Scale: 1/8" = 1'-0"



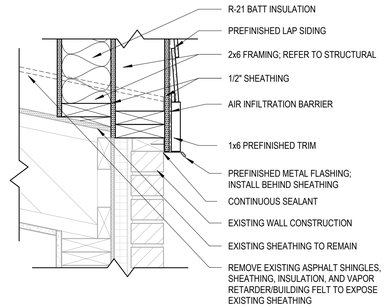
3 Building Section
Scale: 1/8" = 1'-0"



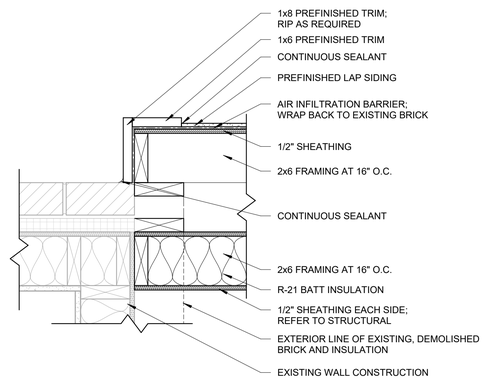
5 Building Section
Scale: 1/8" = 1'-0"



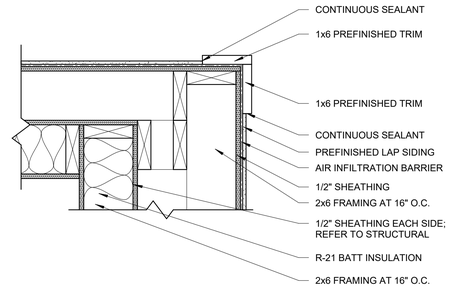
4 Base of Wall at Existing
Scale: 1 1/2" = 1'-0"



6 Base of Wall at Existing
Scale: 1 1/2" = 1'-0"



7 Outside Corner at Existing Wall
Scale: 1 1/2" = 1'-0"



8 Outside Corner at New Wall
Scale: 1 1/2" = 1'-0"

REVISIONS		
MARK	DATE	DESCRIPTION

Contract Documents

February 11, 2026

J2 Project No. J22528

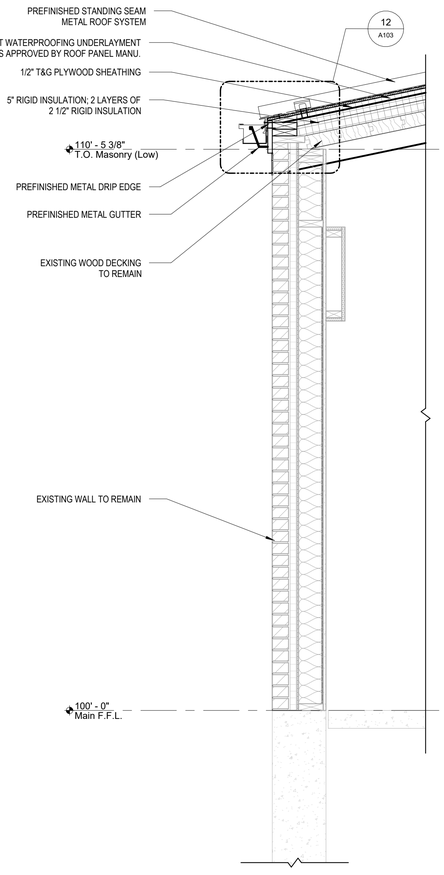
LICENSED ARCHITECT
JAMES DEVINE
(1292)
DATE: 02/11/2026
STATE OF NORTH DAKOTA

City of Bowman
Bowman City Hall Roof Replacement
Bowman, North Dakota

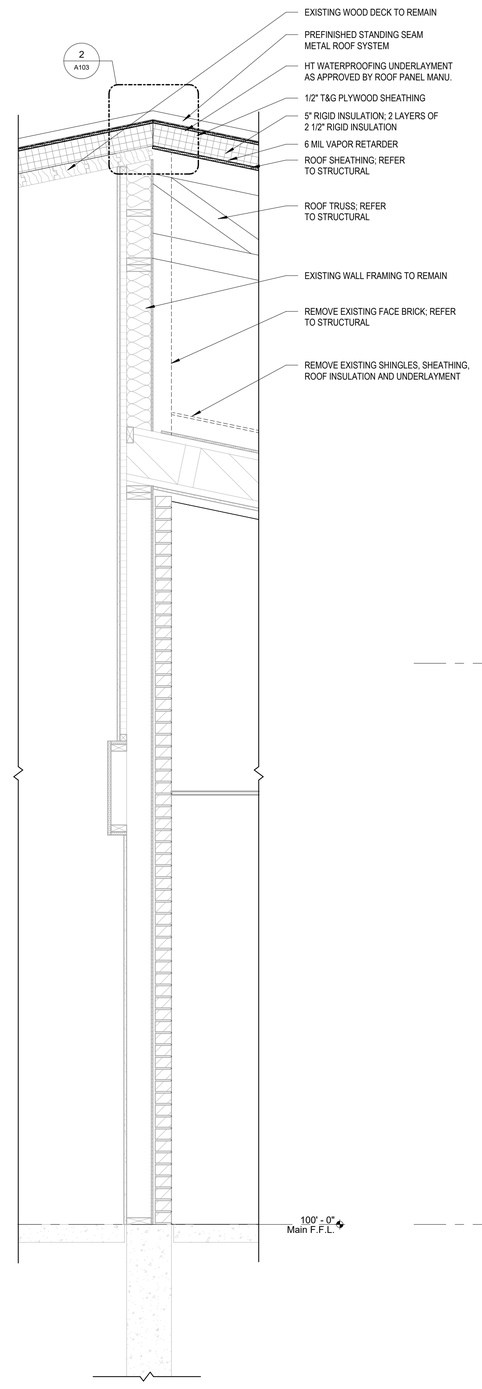
J2 studio
architecture + design, pc
919 South 7th Street, Suite 400
Bismarck, ND 58504
(701) 255-1622

Building Sections

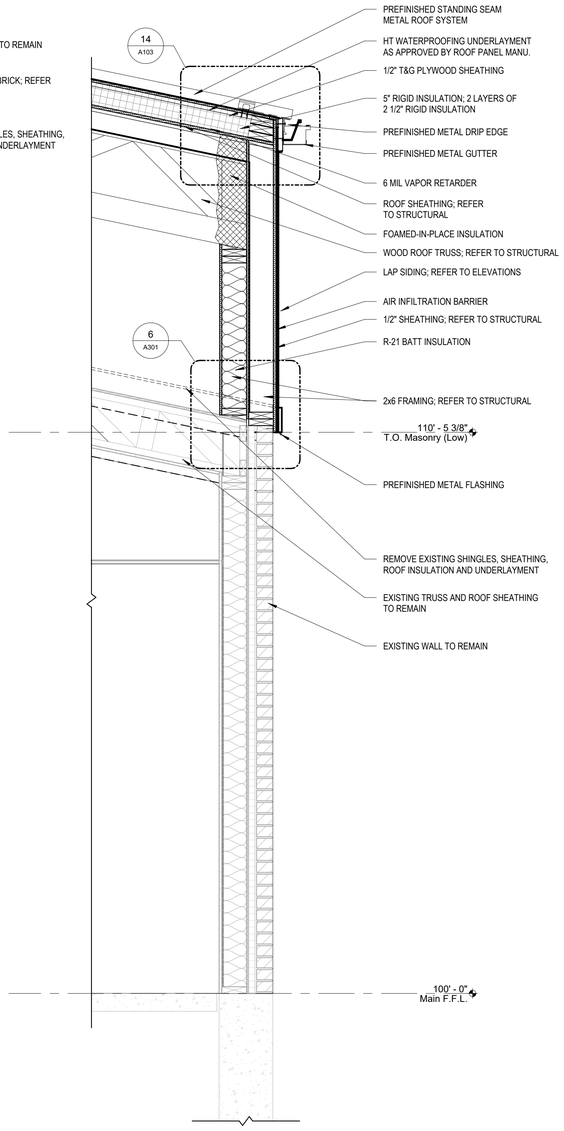
A301



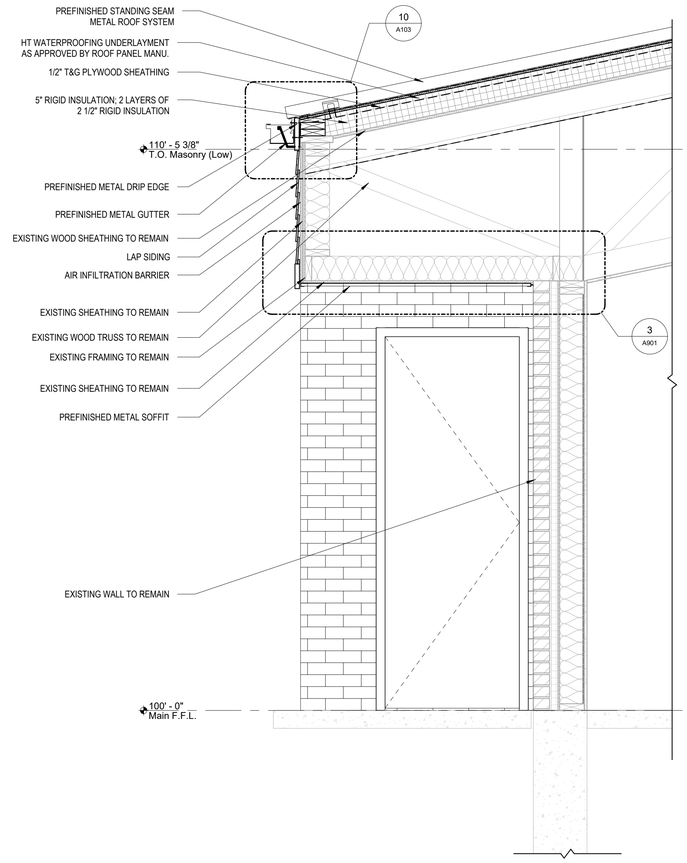
1 Wall Section
Scale: 3/4" = 1'-0"



2 Wall Section
Scale: 3/4" = 1'-0"



3 Wall Section
Scale: 3/4" = 1'-0"



4 Wall Section
Scale: 3/4" = 1'-0"

REVISIONS		
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Contract Documents

February 11, 2026

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JAMES DEVINE
(1292)
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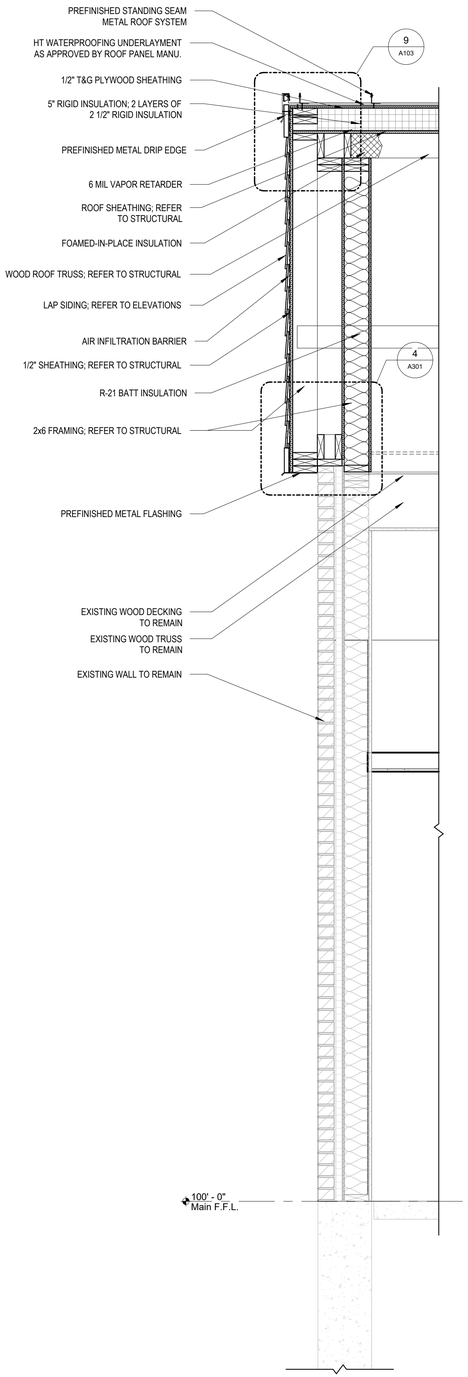
Wall Sections

A302



1 Wall Section
Scale: 3/4" = 1'-0"

2 Wall Section
Scale: 3/4" = 1'-0"

REVISIONS		
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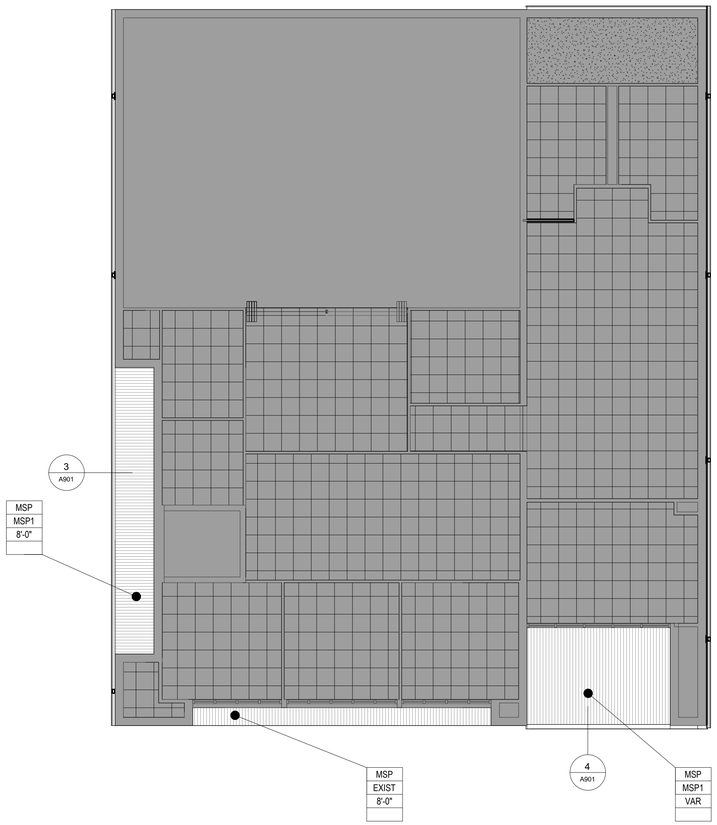
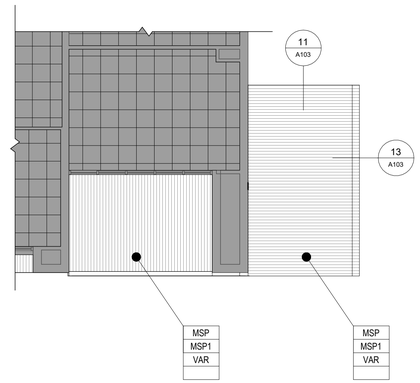
Wall Sections

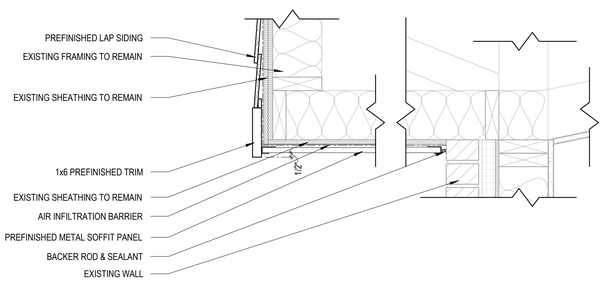
A303



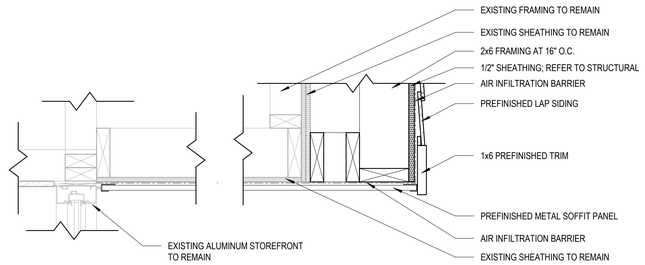
1 Main Floor Reflected Ceiling Plan
Scale: 1/8" = 1'-0"



2 Partial Main Floor Reflected Ceiling Plan (Alt. Bid)
Scale: 1/8" = 1'-0"

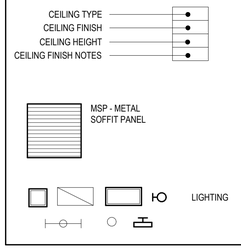


3 Soffit Detail
Scale: 1 1/2" = 1'-0"



4 Soffit Detail
Scale: 1 1/2" = 1'-0"

CEILING FINISH LEGEND:



CEILING FINISHES:

MSP1 - METAL SOFFIT PANEL; PAC-CLAD FLUSH SOFFIT PANEL; COLOR - DARK BRONZE.

CEILING HEIGHT:

VAR - VARIES

CEILING FINISH NOTES:

1. EXISTING SOFFIT TO REMAIN.

REVISIONS

MARK	DATE	DESCRIPTION

Contract Documents

February 11, 2026

J2 Project No. J22528

City of Bowman
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Bismarck, ND 58504
(701) 255-1622

Main Floor & Partial Reflected Ceiling Plans

A901

GENERAL STRUCTURAL NOTES

GENERAL DESIGN AND CONSTRUCTION:

(1) All work shall comply with the 2021 International Building Code (IBC 2021).

(2) Where existing structural information could not be obtained (i.e. wood species, steel grades, foundation sizes, etc.) CWSTRUCTURAL provided analysis of the existing structural components to meet International Existing Building Code (IEBC 2021). Where additional dead or live loads, including snow drift, do not increase the stress on the analyzed structural elements (beam, column, foundation, etc.) more than 5%, those structural elements are considered to be acceptable based upon IEBC.

(3) Design Loads:

Project Location: Minot, North Dakota

Wind loads: Per ASCE 7, Exposure Category "C", Iw = 1.0
Basic wind speed (Mapped 3-second gust) = 112 mph (Risk Category II)

Snow Load: Per ASCE 7 [Also see plan for potential additional snow drifting]
Calculated Roof Snow, Pf = 30 psf
Ground Snow (Mapped), Pg = 35 psf, Is = 1.0, Ce = 1.0, Ct = 1.0

Seismic loads per ASCE 7 (Minot, ND)
Site class = D (Assumed), Ss = 0.066, S1 = 0.022, Ie=1.0
Site coefficient, Fa = 1.6, Site coefficient, Fv = 2.4
Seismic Design Category A

Total Roof Dead Load = 20 psf (superimposed)

(4) Specific notes and details shall take precedence over General Structural Notes.

(5) The contract structural drawings and specifications represent the finished structure. Unless otherwise indicated, they do not indicate the means or method of construction. The contractor is solely responsible for the protection of the structure during all phases of demolition, construction and installation. Provide all measures necessary to protect the structure, workers or other persons by means of shoring, bracing and job site safety measures.

(6) Means-and-methods including temporary bracing and shoring against wind and erection is the responsibility of the contractor.

(7) No area of the structure shall be loaded with construction material or equipment that exceeds final design loading indicated.

(8) Holes, pipes, sleeves, etc. through structural framing and foundations that are not shown on the drawings are not acceptable.

(9) Contractor shall provide a cast-in sleeve where drain tile passes through footings or foundation walls.

(10) Structural engineer's seal on the plan does not provide for construction inspection of any kind.

(11) The cost for additional structural engineering services necessitated by contractor requests for an option or due to errors or omissions in construction shall be the contractor's responsibility.

(12) Shop drawings prepared by suppliers, subcontractors, etc. shall be dimensioned, reviewed, coordinated, and signed/stamped by the general contractor prior to submitting to the structural engineer. Manufactured components such as glulam beams and T&G decking shall be engineered and stamped by a licensed Professional Engineer in the state the project is being built prior to submission and shall include stamped calculations.

(13) Verify all dimensions and conditions of existing construction to be as shown on the drawings. Advise the Architect and Structural Engineer of variances prior to continuing with construction.

(14) Protect existing construction from damage due to construction of new additions. Make no cuts or alterations to existing construction other than those shown on the drawings without the approval of the Architect and Structural Engineer.

(15) CWSTRUCTURAL may provide periodic observations to assure conformance with design intent of the construction documents. However, these observations are not meant to fulfill the requirements of the IBC required special inspections. CWSTRUCTURAL is not considered a qualified "Special Inspector" as it relates to required building code. Refer to the Special Inspections.

STATEMENT OF SPECIAL INSPECTIONS:

(1) Special inspections and structural testing shall be provided by an independent agency employed by the General Contractor or the items identified in this section and in other areas of the approved construction plans and specifications, unless waived by the Building Official (see IBC Chapter 17). All cost for all special inspections required and stated in this section of the General Structural Notes, along with all attached tables, shall be part of the General Contractors Bid. No additional costs may be charged to the owner for the cost of the required special inspections.

(2) Special inspections must be completed by independent agency and report directly to owner or architect. General contractor is required to pay special inspector, however G.C. is not allowed to provided special inspector or testing agency any directive. All reports and correspondence from special inspector or testing agency is to be sent directly to owner and/or architect, not the General Contractor. If any reports are submitted through the contractor to the owner or architect they will need to be resubmitted for approval. At the discretion of the architect, owner, or design team retesting may also be required, at the cost of the special inspector, if reports are not submitted correctly.

(3) The names and credentials of the Special Inspectors to be used shall be submitted to the Building Official for approval.

(4) Duties of the Special Inspector:
- a. The Special Inspector shall review all work listed below for conformance with the approved construction plans and specifications and the IBC.
- b. The Special Inspector shall furnish special inspection reports to the EOR, Contractor, Owner and Building Official on a weekly basis, or more frequently as required by the Building Official. All items not in compliance shall be brought to the immediate attention of the Contractor for correction, and if uncorrected, to the EOR and the Building Official.
- c. Once corrections have been made by the Contractor, the Special Inspector shall submit a final signed report to the Building Official stating that the work requiring special inspection was, to the best of the Special Inspector's knowledge, in conformance with the approved construction plans and specifications as well as the applicable workmanship provisions of the IBC.

(5) Duties and responsibilities of the Contractor:
- a. The Contractor shall submit a written statement of responsibility to the Owner and the Building Official prior to the commencement of work. In accordance with IBC 1704.4, the statement of responsibility shall contain acknowledgement of the special inspection requirements contained within the "Statement of Special Inspections".
- b. The Contractor shall notify the responsible Special Inspector that work is ready for inspection at least one work day (24 hours minimum) before such inspection is required.
- c. All work requiring special inspection shall remain accessible and exposed until it has been observed by the Special Inspector.

(6) **REQUIRED SPECIAL INSPECTIONS (Per IBC 2021)**:

CONCRETE (IBC Section 1705.3): Special inspections to be performed per "Required Special Inspection of Concrete Construction" table located within these construction documents and also called out in the IBC table 1705.3. (Exception: Special inspections are not required for concrete in the following cases:

1. Isolated spread concrete footings of buildings three stories of less above grade plane that are fully supported on earth or rock.
2. Continuous concrete footings supporting walls of buildings three stories or less above grade plane that are fully supported on earth of rock where:
   2.1) The footings supporting walls of light-framed construction
   2.2) The footings are designed in accordance with Table 1809.7 of the IBC.
   2.3) The structural design of the footing is based on a specified compressive strength, f'c, not more than 2,500psi, regardless of the compressive strength specified in the approved construction documents or used in the footing construction.
3) Nonstructural concrete slabs supported directly on the ground, including prestressed slabs on grade, where the effective prestress in the concrete is less than 150 psi.
4) Concrete foundation walls constructed in accordance with Table 1807.1.6.2.
5) Concrete patios, driveways and sidewalks, on grade.)

STRUCTURAL WOOD (IBC Section 1705.11.1 & IBC Section 1705.11.3): *Continuous* special inspection is required during field gluing operations of elements of the main windforce-resisting systems. *Periodic* special inspection is required for nailing, bolting, anchoring, and other fastening of elements of the main windforce-resisting system, including wood shear walls, wood diaphragms, drag struct, braces, and hold-downs. (Exception: Special inspections are not required for wood shear walls, shear panels and diaphragms, including nailing, bolting, anchoring, and other fastening to other elements of the main windforce-resisting system where fastener spacing of the sheathing is more than 4 inches on center.)

WIND-RESISTING COMPONENTS (IBC Section 1705.11.3): *Periodic* special inspection is required for fastening of the following systems and components:
1.) Roof coverings, roof deck, and roof framing connections.
2.) Exterior wall covering, and wall connections to roof and floor diaphragms and framing.(i.e. Wood Studs, Steel Studs, CMU walls, etc.)

SOIL (IBC Section 1705.6): Special inspections to be performed per Table 1705.6 located both in the IBC and within these construction documents.

STRUCTURAL STEEL & STEEL JOISTS (IBC Section 1705.2, 1705.11, & 1705.12, and AISC 360): Special inspections to be performed per "Required Special Inspection of Structural Steel" table located within these construction documents and also called out in the IBC & AISC.

(7) Please see the "Special Inspection Schedules/Tables" for the types, extents and frequency of specific items requiring special inspections and structural tests as part of this project. Inspections and testing indicated in the special inspection tables are guidelines to the required inspection and testing required by the building code for the particular building. The IBC and referenced testing/inspection standards shall be reviewed and followed by the inspecting agency to ensure that all required testing/inspection and procedure for testing/inspection is achieved.

FOUNDATIONS:

(1) Assumed Geotechnical Design Values – All concrete foundation designs are based on assumed geotechnical design values to be confirmed by a Geotechnical Engineer. All foundation excavations must be observed and all assumed design values stated below shall be verified in writing by a licensed Geotechnical Engineer following on-site observation prior to fabricating steel and placing concrete any concrete. All assumed geotechnical design values are as follows:

A. Minimum Soil Bearing Capacity = 2000 psf
B. Backfill Soil Unit Weight (γ) = 110 pcf (dry)
C. Maximum Lateral Pressure Values (Granular Backfill with Draintile):
1. Active Pressure: 41 pcf
2. At-Rest Pressure: 60 pcf
3. Passive Pressure: 250 pcf
4. Coefficient of Friction (Concrete on Soil): 0.35

(2) All footings shall bear on natural, undisturbed soil. All natural bearing material shall be approved in writing by a licensed Geotechnical Engineer prior to placing footing concrete.

(3) All seepage shall be continuously pumped from excavations until the Geotechnical Engineer of Record determines such seepage no longer impacts the bearing soils or Engineered Fill or construction of footings and floor slabs.

(4) Positive drainage shall be obtained away from the structures and care required for backfilling and drainage of utility trenches during construction shall be provided per the Geotechnical Report and the Civil drawings.

(5) Excavations shall be performed in accordance with all governing safety regulations including OSHA. There shall be no surcharge load from vehicles, equipment, materials, soil piles etc. near the crest of the excavation slopes per the Geotechnical Report. The responsibility for excavation safety and temporary construction slopes lies solely with the Contractor.

(6) Clean footing excavations of snow, water, mud, loose soil and debris prior to placing footing concrete.

(7) Footings may not be earth formed.

(8) All footings are centered under the piers, columns or walls they support, except as noted otherwise.

(9) Excavations and backfill shall be executed and tested in accordance with the project specifications and soils report.

(10) Footing excavations shall be to proper line and level to insure minimum concrete cover of footing reinforcement for footing depth.

(11) Backfill shall be compacted by mechanical means.

(12) Backfill shall not be placed against foundations until walls have been adequately braced either temporarily or permanently with new construction fully installed at the top and base of the walls, the concrete has attained sufficient strength to resist lateral pressure and has adequate durability.

(13) Protect all foundations from the action of water and freezing.

(14) See mechanical, electrical and architectural drawings for all openings and inserts not shown on the structural drawings. All openings and inserts shall be placed prior to casting concrete.

(15) Non-frost susceptible granular fill is to consist of granular material with less than 5% passing the #200 sieve and no more than 40% passing the #40 sieve.

CONCRETE:

(1) Concrete mix design(s) shall be by an independent testing laboratory and shall be submitted to the Architect and Structural Engineer for approval at the responsibility of the General Contractor.

(2) Concrete Mix Designs: All cement is to be Portland Limestone Cement (PLC) meeting ASTM C595 with cement type as indicated in the following mix design(s) and with maximum alkali content (NaEq) = 0.60%.

Footings are to have a concrete mix design consisting of Type 1L cement (6 sack min.) and a 28-day compressive strength of 4000 psi, 25% max fly ash allowed, 1 1/2" max aggregate size, 4" max slump, 0.43 max water-to-cement ratio, no air entrainment, water reducing admixture ok.

Concrete Piers are to have a concrete mix design consisting of PLC Type 1L (6 sack min.) and a 28-day compressive strength of 4000 psi, 25% max fly ash allowed, 3/4" max aggregate size, 4" max slump, 0.43 max water-to-cement ratio, 5% to 7% air entrainment, water reducing admixture ok, mid range plasticizer ok, super plasticizer requires prior approval.

Flowable Fill Concrete (Controlled Density Fill - CDF) is to have a minimum compressive strength of 150 psi and a maximum strength of 200 psi and is to consist of water, Portland Lime Cement, fine aggregate and flyash to be self-leveling not needing vibration or compaction with a density of 100 to 125 pcf.

Grout for steel base plates and bearing plates is to be of the non-shrink non-metallic type meeting ASTM C1107 with a maximum slump of 3" and a minimum 28-day compressive strength of 5000 psi. Follow manufacturer's directions for use.

(3) Concrete aggregate shall meet ASTM C33 with a maximum shale or deleterious material content of 1%.

(4) Fly Ash of "Type C meeting Class F" may be substituted by weight for cement up to maximum limits indicated in each concrete mix design.

(5) Mid-Range Plasticizer meeting ASTM C494 Type D is acceptable if noted in mix design. Mix is to have a maximum slump as indicated above for the particular mix design prior to adding the Mid-Range. Adjust air content as required by the supplier due to the use of the Mid-Range and its projected effect on the air content. Test for air after Mid-Range addition to achieve range specified. Slump is to be tested to meet that specified above for particular mix design prior to adding Mid-Range Plasticizer and no additional water may be added after slump test.

(6) Air Entraining agents shall meet ASTM C260.

(7) Concrete construction shall conform to the ACI building code requirements for reinforced concrete, ACI 318.

(8) Hot Weather Concreting per ACI 305R and Cold Weather Placement per ACI 306R shall be followed where weather conditions warrant.

(9) Forms shall be left on all walls for a minimum of 2 days or longer as required at the discretion of the contractor.

(10) A continuous bond break, such as 3/8" asphaltic fiber board expansion joint material, shall be placed between the concrete slab and the perimeter foundation walls.

(11) All concrete pours shall be tested for strength (per ASTM C31 and C39), slump and air content. Test one cylinder at 7 days, one at 14 days, two at 28 days and hold one cylinder.

(12) All concrete reinforcing shall meet ASTM specification A615, Grade 60.

(13) Reinforcing steel shall be bent and placed in accordance with the ACI code. All tension splices shall be class 'B', 40 bar diameters, minimum. All compression splices shall be 40 bar diameters (minimum), unless noted otherwise. Lap all corner bars.

(14) Concrete cover for reinforcing shall be per ACI 318.

(15) All vertical wall and pier bars must be extended to 2" of the top of foundation walls unless shown otherwise on the drawings. All horizontal foundation wall bars must be placed within 4" of the top and bottom of the wall unless shown otherwise.

(16) All epoxy-set rebar doweling to concrete shall be HILTI HIT-HY 200 Safe Set System with HILTI Hollow Drill Bit System, Dewalt AC200+ with DustX + Hollow Drill Bit System, or equal, except as noted otherwise. Install per manufacturer's recommendations.

(17) All adhesive anchor installations in the horizontal to vertically overhead orientation shall be conducted by a certified adhesive anchor installer as certified by ACI per ACI 318-11 17.2.2 or ACI 318-14 17.8.2.2 or ACI 318-19 17.2.3 or approved equivalent. Current ACI certificate must be submitted to the engineer of record for approval prior to commencement of any adhesive anchor installations.

STRUCTURAL STEEL:

(1) Structural steel work is to be per AISC specifications. Steel grade shall be Grade B (Fy=46 ksi) for cold-formed steel tubes and ASTM A36 (Fy=36ksi) for other shapes except as noted otherwise on the drawings.

(2) Columns shall be erected true and plumb. Provide temporary bracing.

(3) Bearing plates for steel columns shall be dry packed with dry grout as specified.

(4) All bolts in structural steel connections shall be ASTM F3125, Group A, standard heavy hex head or twist-off tension control (TC) High-Strength bolts, except as noted otherwise.

(5) All anchor rods for structural steel columns shall be ASTM F1554, Grade 36 (Fy=36ksi) threaded rod with a nut on the embedded end as indicated on the drawings. All anchor rods shall be cast-in-place only – no post installed anchors.

(6) No field welds are to be made until the members are properly aligned. Field welds are to be made by competent certified welders using proper electrodes and amperage.

(7) All exterior exposed structural steel support framing and beam bearing shoes/saddles, etc shall be hot-dip galvanized G90, shop prime painted and shop or field finish painted with exterior weather resistant paint. Paint shall have a minimum applied dry thickness of 6 mils total, which includes primer, mid-coat, and top-coat. Paint over connections and do not disassemble structural framing. Steel fabricator to coordinate shop & field painted coats with contractor.

WOOD AND TIMBER CONSTRUCTION:

(1) All sawn lumber, sheathing, engineered lumber, and timber construction shall conform to job notes and specifications as well as American Wood Council (AWC), and American Institute of Timber Construction (AITC) standards.

(2) All dimensional lumber in contact with concrete shall be treated and shall consist of pressure treated Southern Yellow Pine No. 2 or better with design values per the current ANSI/AF&PA National Design Specification (NDS) Supplement. Design values will vary based on the width of the member.

(3) All dimensional structural framing members including beams, joists, etc (2" to 4" thick x 2" and wider) noted and shown on the plans as "treated" shall be pressure treated Southern Yellow Pine No. 1 grade with the following minimum material properties; bending stress of 1,000 psi, shear parallel to grain stress of 175 psi, compression parallel to grain stress of 1,400 psi, compression perpendicular to grain stress of 565 psi and modulus of elasticity of 1,600,000 psi.

(4) All wood construction shall conform to Chapter 23 of the International Building Code (IBC), except as noted otherwise. All nailing shall conform to Table 2304.10.2 "Fastening Schedule" of the International Building Code unless those requirements noted on the plans and specifications are more strict.

(5) Holes in sawn lumber shall be as specified in the International Building Code (IBC).

(6) Dimension lumber used for headers and jamb members shall be free of checks and splits.

(7) No notching of studs, joists, rafters, beams or trusses is permitted without the engineer's approval. Holes bored in the stud or joist shall be in the middle one-third of the depth and the diameter of any such hole shall not exceed one-fourth the depth of the member.

(8) Fastening of structural wood members shall be as specified and as detailed.

(9) All construction adhesive for wood-to-wood, wood-to-steel, wood-to-concrete shall meet Performance Specifications APA AFG-01 and ASTM D-3498 and shall have a minimum shear strength at 28 days of 400 psi. G.C. to follow all manufacturer's recommendations for adhesive installation time, temperature, conditions, and coverage, provided that the full surface area of both members/surfaces to be bonded are covered w/ adhesive as recommended.

(10) All fasteners including nails, screws, anchor bolts, etc. and connectors in contact with treated lumber or sheathing shall be hot-dip galvanized with a minimum G90 coating. Including all fasteners from wall studs to treated wall sill plates.

(11) All exterior exposed framing connectors and fasteners per IBC 2304.10.5 must meet ASTM A653, Type G185 zinc-coated galvanized steel, stainless steel or equivalent.

(12) All Laminated Veneer Lumber (LVL) shall be Pacific Wood Tech PWT treated and shall have an allowable bending stress of 3,100 psi, compression perpendicular to grain stress (parallel to glue line) of 850 psi, compression parallel to grain stress of 2,900 psi, horizontal shear stress perpendicular to glue line of 285 psi, and modulus of elasticity of 2,000,000 psi.

TRUSSES:

(1) Roof trusses shall be designed and fabricated by a "Truss Plate Institute" (TPI) member fabricator and shall be fabricated to TPI standards.

(2) Truss fabricator to design and show size and location of permanent and erection bracing perpendicular to trusses on truss shop drawings (to be installed by general contractor).

(3) The Contractor shall be responsible for all erection, handling, installation, restraint and bracing of Metal Plate Connected Wood Trusses per the current Building Component Safety Information (BCSI) by SBCA and TPI as well as the current HIB-91 (Commentary and Recommendations For Handling Installing & Bracing Metal Plate Connected Wood Trusses by the Truss Plate Institute (TPI).

(4) Roof trusses are to be designed for 30 psf snow load and 20 psf dead load at top chords and 10 psf dead load at bottom chords.

(5) Roof trusses are to be designed for L/360 live load deflection and L/240 total load deflection.

(6) Truss member overall dimensions, bearing points, top and bottom chord configuration, bearing heel height and bearing points shall be as shown on the contract documents. Overall truss design including web member configuration, member sizes, connector plates, etc. by truss fabricator shall be based on loads indicated.

(7) Truss fabricator to furnish shop drawings including proposed truss member design, overall layout plan and lateral bracing to be completed and sealed by a licensed professional engineer in the state the project is being built.

(8) Handling of trusses and erection bracing is the responsibility of the contractor. The contractor shall provide temporary and permanent diagonal lateral and cross bracing until roof sheathing, ceilings, and permanent bracing can be applied and shearwalls completed.

(9) Truss supplier shall provide design of all permanent and temporary lateral bracing of all truss members as required to maintain strength, stability and serviceability of trusses to support the required loads under construction as well as in service.

(10) Provide solid blocking between trusses at bearings and along hip and ridge lines. Nail deck to blocking at ridge and hip lines. Provide intermediate bridging as required by truss design.

HORIZONTAL WOOD STRUCTURAL PANEL SHEATHING (ROOFS):

(1) ROOF SHEATHING DIAPHRAGM – All exterior roof sheathing shall be a minimum of 19/32" Exposure 1 rated plywood sheathing (with a 24/16 APA Span Rating) conforming to DOC PS 1, DOC PS 2, or CSA O437 or CSA O325. All panels shall be identified with a grade mark or certificate of inspection issued by an APA approved agency. Panels are to be unblocked except that edges of all panels shall be supported on truss, joist, or blocking at roof edges as detailed. Sheathing shall be fastened with 10d common or galvanized box nails (galvanized nails shall be hot-dipped or tumbled) at 6" o.c. at panel edges and at 12" o.c. at intermediate framing members. Nails are to have a minimum penetration of 1 1/2" into framing members and are to be located not less than 3/8" from edges of panels and framing.

(2) All roof panel sheathing shall be oriented with the strength axis perpendicular to the supports.

(3) Roof sheathing panels shall be continuous over three or more spans (minimum).

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Contract Documents

Febuary 11, 2026

J2 Project No. J22528
CW PROJECT NO. C2570P

CHRIS K. WENTZ REGISTERED PROFESSIONAL PE 4086 ENGINEER NORTH DAKOTA
2/11/2026

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Bowman City Hall Roof Replacement
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GENERAL STRUCTURAL NOTES

S001

REQUIRED SPECIAL INSPECTIONS OF CONCRETE CONSTRUCTION

(IBC Table 1705.3)

	FREQUENCY		
	Continuous	Periodic	Comments:
1) Inspect Reinforcement			
Inspect reinforcement, including prestressing tendons, and verify placement.		♦	Verify reinforcing and placement as shown on drawings and per ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3 & IBC 1908.4
2) Reinforcing Bar Welding:			
Verify weldability of reinforcing bars other than ASTM A706:		♦	Verify bar welding per AWS D1.4 & ACI 318: 26.6.4
Inspect single-pass fillet welds, maximum 5/16th; and		♦	
Inspect all other welds.	♦		
3) Inspect anchors cast in concrete.		♦	Verify anchors are cast in concrete per structural drawings and ACI 318: 17.8.2
4) Inspect anchors post-installed in hardened concrete members b:			
Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads.	♦		Verify anchors are post installed in concrete per structural drawings, ACI 318: 17.8.2.4 (adhesive anchors) & 17.8.2 (mechanical anchors)
Mechanical anchors and adhesive anchors not defined in 4.a.		♦	
5) Verify use of required design mix.		♦	Verify mix designs meet strength and exposure requirements listed on approved plans; verify compliance with ACI 318: Ch. 19, 26.4.3, 26.4.4, & IBC 1904.1, 1904.2, 1908.2, 1908.3
6) Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	♦		Verify, perform, & examine tests per ASTM C172, ASTM C31, ACI 318: 26.4 & 26.12, and IBC 1908.10
7) Inspect concrete placement & application techniques.	♦		Inspect concrete & shotcrete placement for proper application per ACI 318: 26.5 & IBC 1908.6, 1908.7, & 1908.8
8) Verify curing temperature and techniques.		♦	Verify concrete curing temperature and technique is being maintained per approved plans, ACI 318: 26.5.3-26.5.5, & IBC 1908.9
9) Inspect prestressed concrete for:			
Application of prestressing forces: and	♦		Verify prestressed concrete per ACI 318: 26.10
Grouting of bonded prestressing tendons.	♦		
10) Inspect erection of precast concrete.		♦	Inspect erection per ACI 318: Ch 26.8
11) Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.		♦	Verify strength per ACI 318: 26.11.2
12) Inspect formwork for shape, location and dimensions of the concrete member being formed.		♦	Inspect formwork for shape, locations, and dimensions of concrete member being formed per approved drawings and ACI 318: 26.11.1.2(b)

For SI: 1 inch = 25.4 mm.

(a) Where applicable, see also Section 1705.12, Special inspections for seismic resistance.

(b) Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with i7.8.2 in ACI 318, or other qualification procedures. Where specific requirements are not provided, special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

(IBC 2015 Section 1705.6 & Table 1705.6)

	FREQUENCY		
	Continuous	Periodic	Comments:
1) Minimum Testing:			
Verify Materials below shallow foundations are adequate to achieve the design bearing capacity.		♦	
2) Prior to Construction:			
Verify excavations are extended to proper depth and have reached proper material.		♦	
3) As Construction Begins:			
Perform classification and testing of compacted fill material.		♦	
4) During Construction:			
Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	♦		
Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.		♦	

REQUIRED SPECIAL INSPECTIONS OF STRUCTURAL STEEL

	FREQUENCY		
	Continuous	Periodic	Comments:
OTHER THAN STRUCTURAL STEEL (IBC 1705.11.3)			
Steel Roof & Floor Deck: (SDI -QA/QC)			
Material verification of steel deck		♦	Identification markings per applicable ASTM standard.
Roof and Floor deck welds		♦	Verify that welds conform to AWS D1.3.
Installation of mechanical fasteners		♦	Verify that fasteners conform to SDI C, SDI NC, SDI RD & manufacturer's instruction as well as construction documents
STRUCTURAL STEEL CONSTRUCTION (IBC 1705.2, 1705.11, 1705.12)			
Prior to Welding (Table N5.4-1, AISC 360-16):			
Verify welding procedures	♦		
Material identification		♦	Verify type and grade of material.
Welder identification		♦	Verify there is a system in place to identify the welder who has welded a joint or member.
Fit-up groove welds		♦	Verify joint preparation, dimensions, cleanliness, tacking and backing.
Access holes		♦	Verify configuration and finish.
Fit-up fillet welds		♦	Verify alignment, gaps at root, cleanliness of steel surfaces, tack weld quality and location.
During Welding (Table N5.4-02, AISC 360-16):			
Use of qualified inspectors		♦	Verify that welders are appropriately qualified.
Control and handling of welding consumables		♦	Verify packaging and exposure control.
Cracked tack welds		♦	Verify welding is not over a cracked tack weld.
Environmental conditions		♦	Verify wind speed is within limits as well as precipitation and temperature.
WPS followed		♦	Verify items such as welding equipment settings, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position.
Welding techniques		♦	Verify interpass and final cleaning, each pass is within profile limitations, and quality of each pass.
Steel headed studs anchors atop beams	♦		Verify placement and installation of steel headed stud anchors. See general structural notes for additional commentary of the testing of headed stud anchors
After Welding (Table N5.4-3, AISC 360-16):			
Welds cleaned		♦	Verify that welds have been properly cleaned.
Size, length and location of welds	♦		
Welds meet visual acceptance criteria	♦		
Arc strikes	♦		
k-area	♦		
Backing & welding tabs removed	♦		
Repair activities	♦		
Document acceptance/rejection of weld	♦		
No prohibited welds have been added without the approval of the EOR		♦	
Nondestructive Testing (N5.5, AISC 360-16):			
CIP welds (Risk Cat. II)		♦	Ultrasonic testing shall be performed on 10% of CJP groove welds in butt, T-and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects.
CIP welds (Risk Cat. III or IV)			Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects.
Access holes (flange > 2")	♦		
Welded Joints subject to fatigue	♦		
Other Steel Inspections (Table N5.7, AISC 360-16; Tables J8-1 and J10-1, AISC 341-10):			
Structural steel details		♦	All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans.
Anchor rods/embeds supporting structural steel		♦	Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete.
Reduced beam sections (RBS)		♦	Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341).
Protected Zones		♦	Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341)
OPEN-WEB STEEL JOIST AND JOIST GIRDERS (2015 IBC Section 1705.2.3 & TABLE 1705.2.3)			
Installation of Open-Web Steel Joists and Joist Girders			
End connections – welding or bolted.		♦	SJI specifications listed in Section 2207.1.
Bridging – horizontal of diagonal.			
Standard bridging.		♦	SJI specifications listed in Section 2207.1.1
Bridging that differs from the SJI specifications listed in Section 2207.1.1.		♦	

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SPECIAL INSPECTION TABLES

S002

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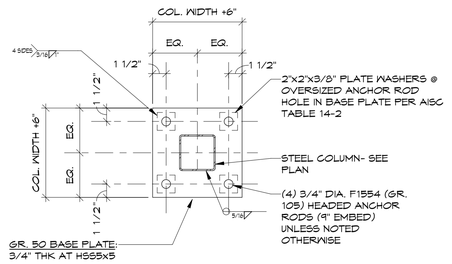
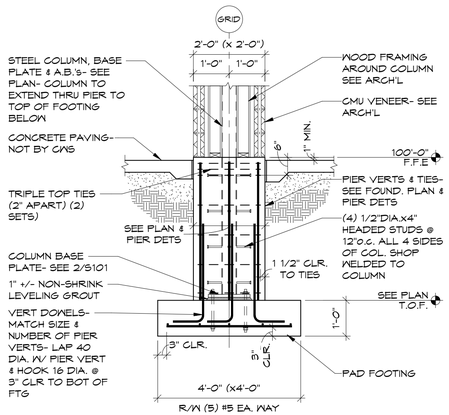


TYPICAL CENTERED HSS COLUMN BASE PLATE

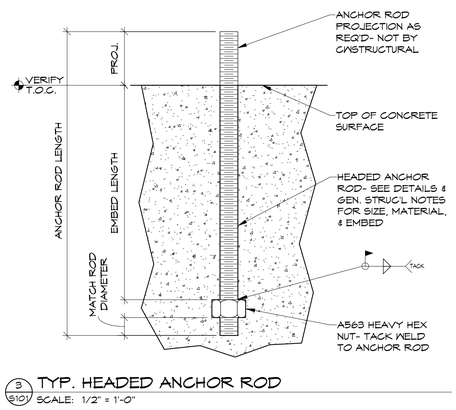
2 S101 "TYPE 1" COLUMN BASE PLATE

SCALE: 3/4" = 1'-0"



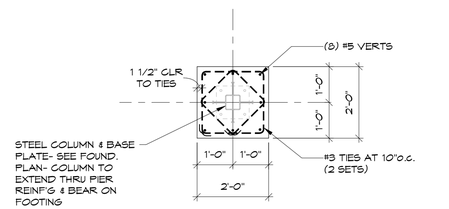
1 S101 EXTERIOR PIER FOUND. DETAIL MAIN ENTRY

SCALE: 1/2" = 1'-0"



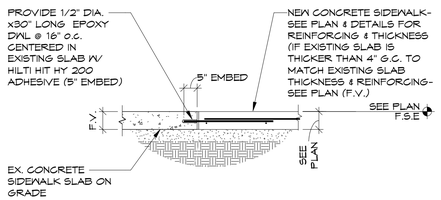
3 S101 TYP. HEADED ANCHOR ROD

SCALE: 1/2" = 1'-0"



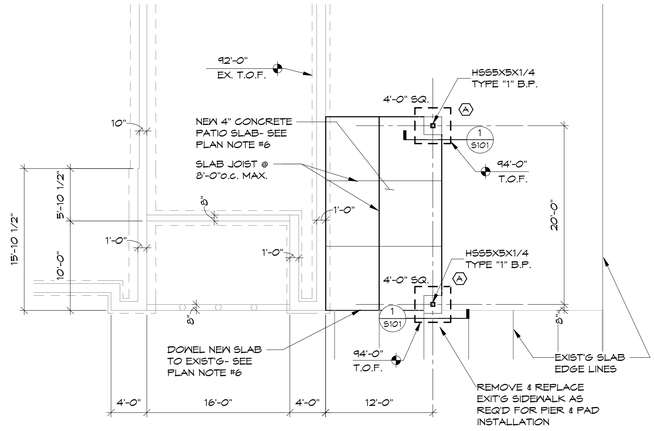
A EXTERIOR PIER DETAIL

SCALE: 1/2" = 1'-0"



4 S101 TYP. NEW SLAB DOWEL TO EX. SLAB

SCALE: 1/2" = 1'-0"



ANOPY FOUNDATION PLAN- ALT. G1

SCALE: 1/8" = 1'-0"

NORTH

PLAN NOTES:

(1) ALL PAD FOOTINGS ARE CENTERED BELOW THE COLUMN THEY SUPPORT - SEE FOUNDATION PLAN & PAD FOOTING SCHEDULE FOR SIZE & REINFORCING. SEE TYPICAL FOUNDATION DETAILS FOR COLUMN BASE PLATE INFORMATION.

(2) 1XX'-0" T.O.F. INDICATES TOP OF FOOTING ELEVATION.

(3) "EX." & "EXIST'G" INDICATES EXISTING CONSTRUCTION. EXIST'G MEMBER SIZES & DIMENSIONS ARE ASSUMED- FIELD VERIFY AS REQ'D.

(4) "F.V." INDICATES THAT DIMENSION IS TO BE FIELD VERIFIED PRIOR TO MATERIAL FABRICATION, CONCRETE CONSTRUCTION, OR STEEL FABRICATION ETC...

(5) X INDICATES PIER DETAIL- REFER TO PIER DETAILS LOCATED ON SHEET S101 FOR PIER SIZE AND REINFORCING

(6) 4" CONCRETE SLAB ON GRADE R/W #4's @ 24"O.C. EA. WAY- SEE G.S.N. FOR SLAB BASE. DOWEL TO EXIST'G SLAB ON GRADE PER DETAIL 4/S101.

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ALT. CANOPY FOUNDATION PLAN & DETAILS

S101

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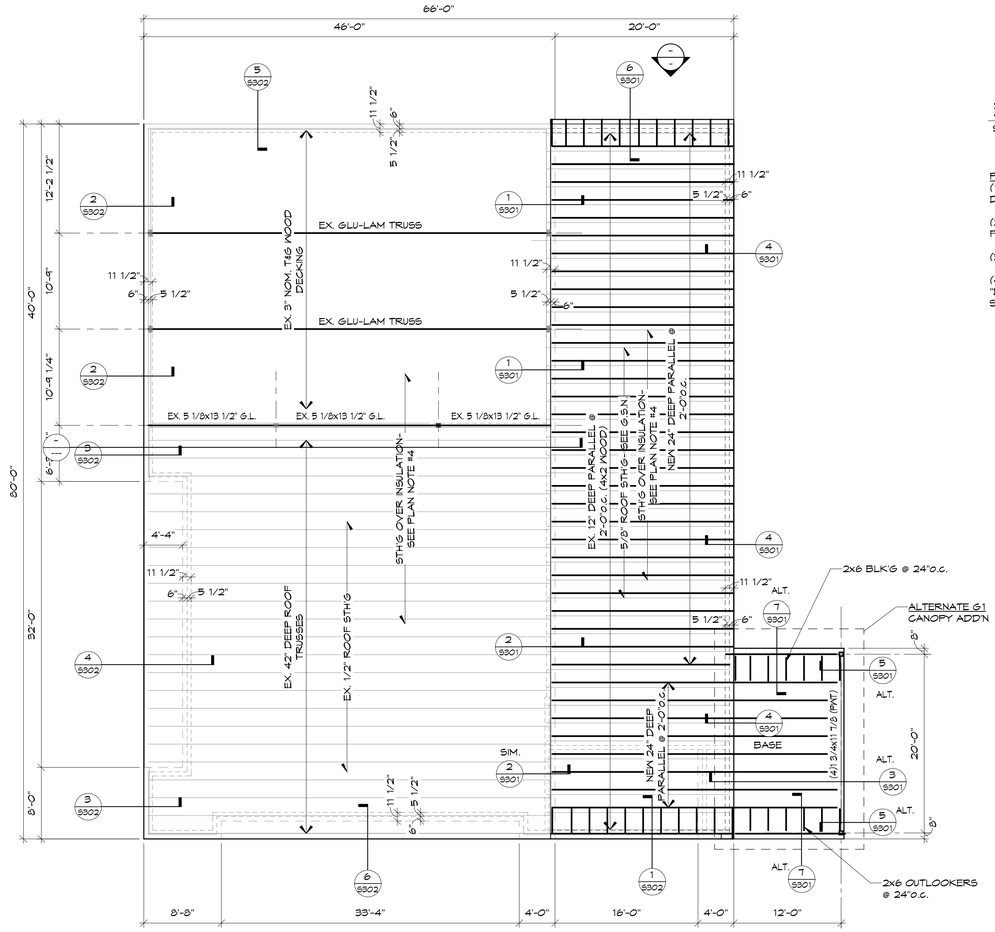

EXIST'G/ NEW ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

PLAN NOTES:
(1) "EX." & "EXIST'G" INDICATES EXISTING CONSTRUCTION. EXIST'G MEMBER SIZES & DIMENSIONS ARE ASSUMED- FIELD VERIFY AS REQ'D.

(2) "F.V." INDICATES THAT DIMENSION IS TO BE FIELD VERIFIED PRIOR TO MATERIAL FABRICATION, CONCRETE CONSTRUCTION, OR STEEL FABRICATION ETC...

(3) CANOPY ON EAST SIDE IS ALTERNATE G1

(4) ROOF SHEATHING OVER INSULATION ANCHORAGE TO ROOF- SEE ARCH'L. "DO NOT" SCREW THRU T&G WOOD DECKING & ANCHORS OVER THE REMAIN ROOF SHOULD TRY ANCHORING TO ROOF TRUSSES.

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EXIST'G/NEW ROOF OVERBUILD FRAMING PLAN

S201

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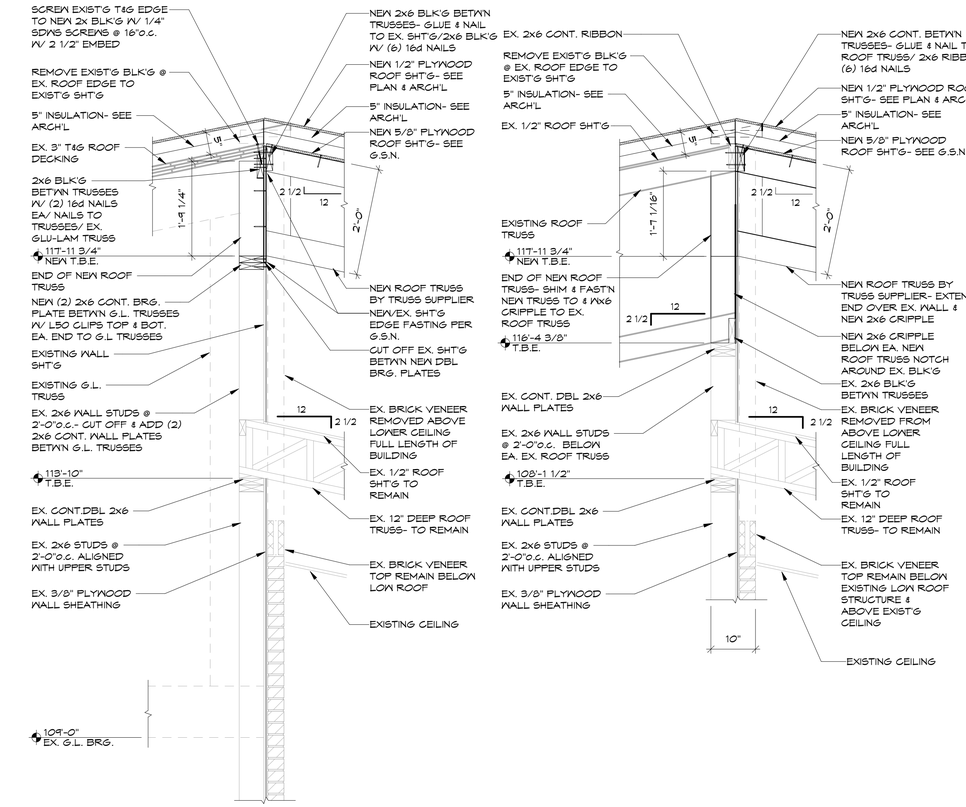
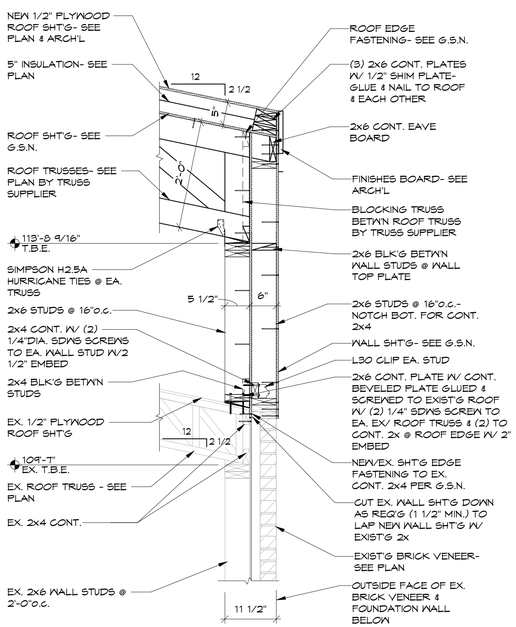
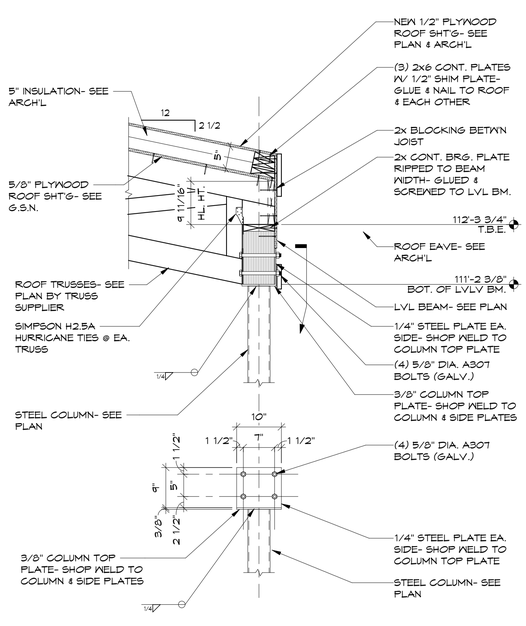


1 (S301) EXISTING & NEW ROOF TRUSS BRG. DETAIL
SCALE: 3/4" = 1'-0"



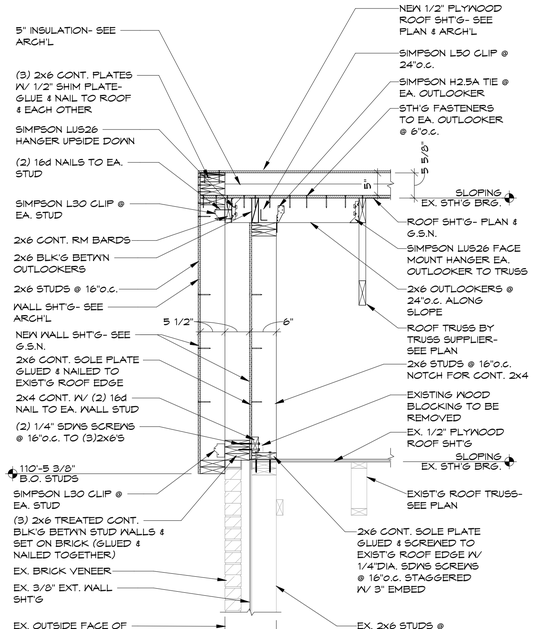
4 (S301) EXIST'G ROOF EDGE W/ OVERBUILD TRUSS BRG.
SCALE: 3/4" = 1'-0"

2 (S301) EXISTING & NEW ROOF TRUSS BRG. DETAIL
SCALE: 3/4" = 1'-0"

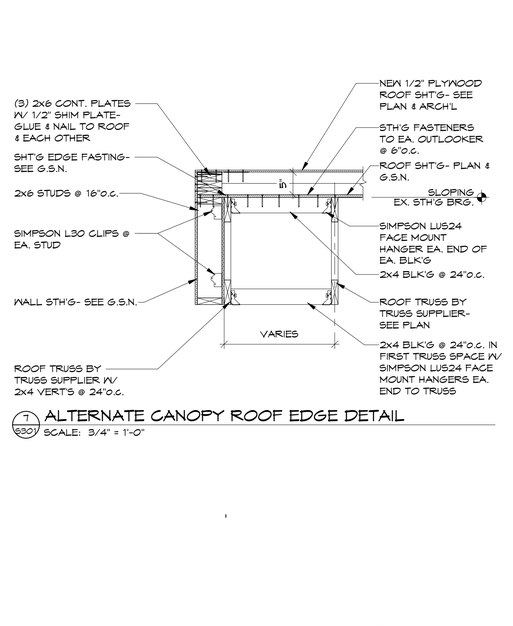


5 (S301) ALTERNATE ROOF BEAM BEARING ON COLUMN
SCALE: 3/4" = 1'-0"

3 (S301) ALTERNATE G1 EXIST'G ROOF EDGE W/ OVERBUILD TRUSS @ CANOPY
SCALE: 3/4" = 1'-0"



6 (S301) EXIST'G ROOF EDGE @ OVERBUILD ROOF
SCALE: 3/4" = 1'-0"



7 (S301) ALTERNATE CANOPY ROOF EDGE DETAIL
SCALE: 3/4" = 1'-0"

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City of Bowman
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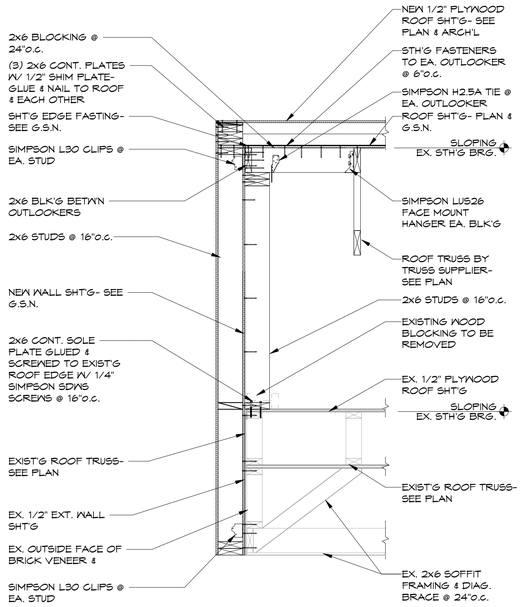
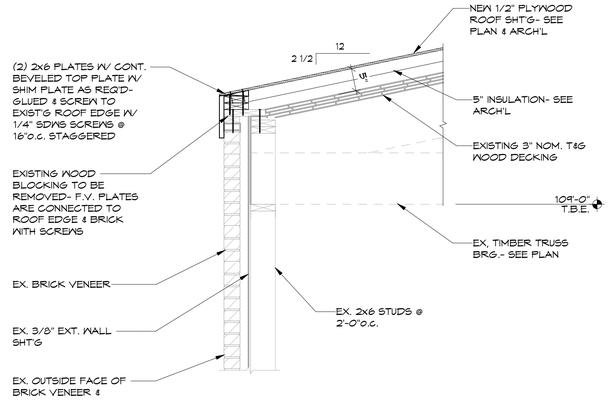
ROOF FRAMING DETAILS

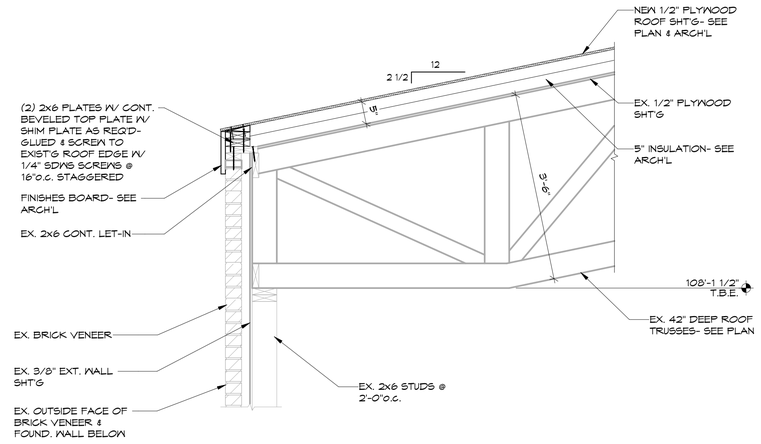
S301



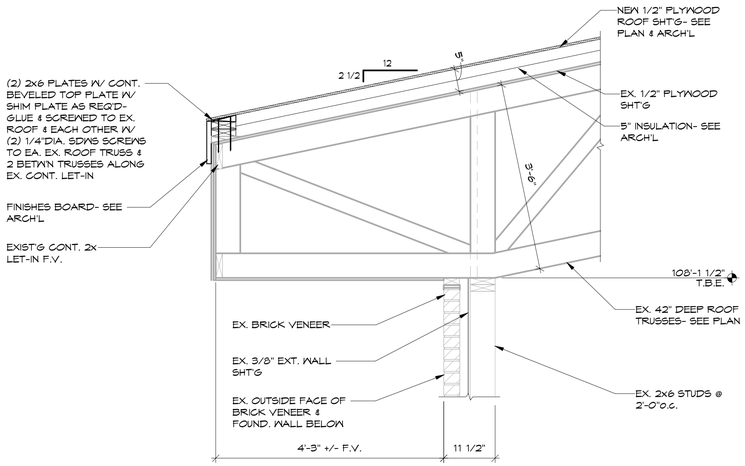
1 E302 EXIST'G ROOF EDGE @ OVERBUILD ROOF @ ENTRY
SCALE: 3/4" = 1'-0"



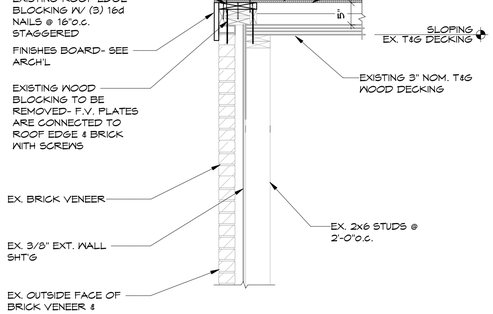
2 E302 EXIST'G ROOF EDGE @ G.L. TRUSS BEARING
SCALE: 3/4" = 1'-0"



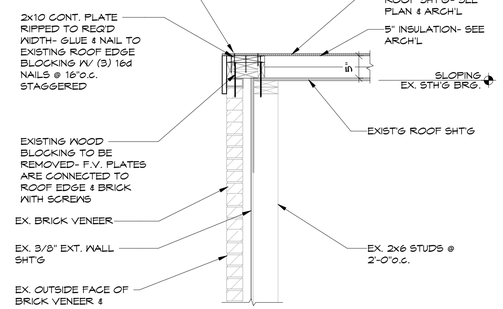
3 E302 EXIST'G ROOF EDGE @ TRUSS BEARING
SCALE: 3/4" = 1'-0"



4 E302 EXIST'G ROOF EDGE @ TRUSS CANT. BEARING
SCALE: 3/4" = 1'-0"



5 E302 EXIST'G ROOF EDGE @ T&G DECK BRG.
SCALE: 3/4" = 1'-0"



6 E302 EXIST'G ROOF EDGE @ ROOF SHT'G BEARING
SCALE: 3/4" = 1'-0"

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ROOF FRAMING DETAILS

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