

**ADDENDUM 01**

Addendum #	01	Date Issued	August
Project Name Job #	20263400 New Town Grandstands		20263400
Bid Date Time	September 10, 2026		2:00pm CDT

THIS ADDENDUM AMENDS AND BECOMES PART OF THE CONTRACT DOCUMENTS FOR EAPC PROJECT 20263400 DATED 08/17/2026, RESPECTIVELY. EACH BIDDER SHALL ACKNOWLEDGE RECEIPT OF THIS ADDENDUM BY MARKING THE ADDENDUM NUMBER AND DATE ON THE BID FORM.

SPECIFICATIONS

- 00 1100 DELETE the following sections from the Table of Contents, they are not required for the project.
- 01 2300 - Alternates
 - 08 4313 - Aluminum-framed Storefronts
 - 09 9113 - Exterior Painting
 - 09 9123 - Interior Painting
 - 10 5613 - Metal Storage Shelving
 - 12 2400 - Window Shades
 - 21 0523 - General-Duty Valves for Water-Based Fire Suppression Piping
 - 21 0553 - Identification for Fire Suppression Piping and Equipment
 - 21 1300 - Fire Suppression Sprinkler Systems
 - 23 3600 - Air Terminal Units
- 00 2000 Geotechnical Report is available. See attached.
- 00 3000 REVISE Bid Form to include Contingency Allowance line. See attached.
- 01 2100 ADD the section 01 2100 - Allowances. See attached.

DRAWINGS

- S001 STRUCTURAL GENERAL NOTES
- 1. EARTHWORK AND SUBGRADE PREPARATION – Geotechnical report reference added.
- S002 STRUCTURAL LOADS, MTL NOTE TABLES AND SPECIAL INSPECTION TABLES
- 1. Clarification added and notes changed.
- S201 LOWER-LEVEL FOUNDATION/SLAB PLAN
- 1. Keynotes changed.
 - 2. Foundation elevations adjusted.
- S202 MAIN LEVEL FRAMING PLAN
- 1. Keynotes changed and added.
 - 2. Plan details added or modified.



ADDENDUM 01 *continued*

S203	PRESS BOX FLOOR FRAMING PLAN
	1. Keynotes changed and added.
	2. Plan details added or modified.
S204	ROOF FRAMING PLAN
	1. Keynotes and notes changed.
S401	FRAMING ELEVATIONS
	1. Elevation overhang ends changed.
S601	FOUNDATION DETAILS
	1. Details changed and added.
S602	FOUNDATION DETAILS
	1. Details changed.
S701	FRAMING DETAILS
	1. Details changed and added.
A403	ENLARGED BUILDING SECTIONS & SIGNAGE, PERFORATED WALL DETAILS
	1. Details added.
	2. Dimensions revised/added.
A441	WALL SECTIONS
	1. Detail revised.
A521	STAIR PLANS AND SECTIONS, DETAILS
	1. Detail revised.
A902	FINISH PLAN
	1. Plan revised.
A921	INTERIOR FINISH DETAILS & MATERIAL IDENTIFICATION CODES
	1. Finish Legend revised.

PRIOR APPROVALS

SECTION	ITEM(S)	APPROVED MANUFACTURER / PRODUCT
08 5113	Aluminum Windows	EFCO
09 5423	Linear Metal Ceilings	USG Planx Lumabuilt Mosaic

ATTACHMENTS

Plan Holders List
Prebid Meeting notes

00 2000 – Geotechnical Report
00 3000 – Bid Form
01 2100 – Allowances

S001 STRUCTURAL GENERAL NOTES
S002 STRUCTURAL LOADS, MTL NOTE TABLES AND SPECIAL INSPECTION TABLES
S201 LOWER-LEVEL FOUNDATION/SLAB PLAN
S202 MAIN LEVEL FRAMING PLAN

**ADDENDUM 01** *continued*

S203	PRESS BOX FLOOR FRAMING PLAN
S204	ROOF FRAMING PLAN
S401	FRAMING ELEVATIONS
S601	FOUNDATION DETAILS
S602	FOUNDATION DETAILS
S701	FRAMING DETAILS
A403	ENLARGED BUILDING SECTIONS & SIGNAGE, PERFORATED WALL DETAILS
A441	WALL SECTIONS
A521	STAIR PLANS AND SECTIONS, DETAILS
A902	FINISH PLAN
A921	INTERIOR FINISH DETAILS & MATERIAL IDENTIFICATION CODES

END ADDENDUM 01



PLAN HOLDERS LIST
New Town Grandstands
New Town, ND
EAPC Project: # 20263400

BID DATE: Thursday, September 10, 2026
BID TIME: 2:00 PM CDT

PLAN HOLDER	CONTACT PERSON	ADDRESS	CITY	STATE	ZIP	PHONE	FAX	EMAIL
EAPC								
BUILDERS EXCHANGES								
Aberdeen Builders Exchange		302 N. Jackson Street	Aberdeen	SD	57401	605-225-4733	605-225-4733	dakotabuild@midconetwork.com
Billings Builders Exchange		2050 Broadwater Avenue	Billings	MT	59102	406-652-1311	406-652-1391	bbx@billingsplanroom.com
Bismarck Builders Exchange		215 Airport Road	Bismarck	ND	58504	701-258-4215	701-258-1391	info@bbxnd.com
Bismarck-Mandan Builders Exchange		P. O. Box 550	Mandan	ND	58554	701-667-4322	701-667-5217	bismandblderex@gmail.com
CMD (Construction Market Data)		Suite 500	Norcross	GA	30092	770-417-4000	800-508-5370	content@constructconnect.com
Construction Industry Center		Box 1227	Rapid City	SD	57709	605-343-5252	605-343-4591	cic@constructionindustrycenter.com or chrissy@constructionindustrycenter.com
Duluth Builders Exchange		802 Garfield Avenue	Duluth	MN	58601			see other note
Fargo-Moorhead Bldrs. Exchange		P. O. Box 10076	Fargo	ND	58106	701-237-6772	701-232-1653	upload
Grand Forks Builders Exchange		2211 S. Washington St. Ste D	Grand Forks	ND	58201	701-772-7798	701-772-0387	planroom@bntgf.com
McGraw Hill Const./Dodge Plan Rm.		1401 Glenwood Avenue No.	Minneapolis	MN	55405	612-381-2290	612-381-2295	dodge.docs@construction.com
Minnesota Builders Exchange		1123 Glenwood Avenue	Minneapolis	MN	55405	612-381-2620	612-381-2621	new project: jb@mbex.org addendums: addenda@mbex.org
Minot Builders Exchange		2424 Burdick Expy East	Minot	ND	58701	701-838-5353	701-839-0638	mbe1@srt.com
Plains Builders Exchange		220 N. Kiwanis Ave.	Sioux Falls	SD	57104-2530	605-334-8886	605-334-0112	info@plainsbuilders.com
QuestCDN.com								www.questcdn.com
St. Cloud Builders Exchange		110 6th Ave S / P.O. Box 746	St. Cloud	MN	56301	320-252-5832	320-251-0081	see other note
Sioux Falls Builders Exchange		1418 "C" Avenue	Sioux Falls	SD	57104	605-357-8687	605-357-8655	info@sfbx.com
Williston Builders Exchange		2108 - 4th Ave. West	Williston	ND	58801-3423	701-572-9460	701-572-2511	willistonbuildersexchange@gmail.com
CONTRACTOR								
Main & Holmes Electric	Lona Thiesen	2626 Valley Street	Minot	ND	58701	701.852.4445		jonat@main-holmes.com
Maertens-Brenny Construction Co		8251 Main St NE	Minneapolis	MN	55432	763.786.4779		info@maertensbrenny.com
Corland Construction	Jon Kleinke	1175 Red River Dr	Sidney	MT	59270	406.433.8158		jon@corlandconstruction.com
Dig It Up Backhoe Service Inc	Travis Bohl	800 163rd Ave SE	Minot	ND	58701	701.722.3315		digitup@srt.com
Tooz Constuction	James Donivant		Dickinson	ND		701690-1675		james@toozconstruction.com
First Dakota Enterprises	Archie Baumann		Fort Pierre	SD		605-223-9600		archie.baumann@firstdakotaenterprises.com
Fed Serve LLC	Melanie Foley		West Fargo	ND		701.361.0541		melanie@fedservllc.com
Missouri River Contracting Inc	Tim Miller		Bismarck	ND		701.258.9140		tim@missouririvercontracting.com
Hight Construction	Jayden Larson		Minot	ND		701-839-5819		jayden@hightconstruction.com
DLC Construction	Lucas Brown		Minot	ND	58702	701-837-0190		lucasbrown81@yahoo.com
Consolidaetd Construction Co. Inc.	Cassidy Hartman		Bismarck	ND	58504	701-806-1333		chartman@1call2build.com
Mattson Construction	Bob Testa		Minot	ND		701-852-3293		bobt@mattson.co
Excavating Inc	Jeff Mitchel	5820 Hwy 2 E	Minot	ND	58701	701.839.1817		mailbox@excavatinginc.com
Dakota Fence/3D Specialties	Jon Pepple	2060 Sheyenne St	West Fargo	ND	58078	701.237.6181		jpepple@dakotaplayground.com
SUPPLIER								
Midwest Glass Reps	Kayla Fore	PO Box 184	Albertville	MN	55301	763.497.4865		kayla@midwestglassreps.com
Quality Flow Sys, Inc	Pat Malay	800 6th St NW	New Prague	MN	56071	952.758.9445		joshm@gfsi.net
Core and Main LP	Heather Schumacher	1830 Craig Park Ct	St. Louis	MO	63146	314.432.4700		heather.schumacher@coreandmain.com



MEETING AGENDA

Date Time	August 24, 2026	2:00pm CDT
Project Name Job #	New Town School Grandstands	20263400
Meeting Type	Prebid Meeting	

AGENDA

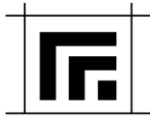
A. Bid Information:

1. Bid Date & Time: 2:00pm CDT, Thursday, September 10, 2026
2. Bid Location: New Town High School Board Room
3. Bid Bond: 5% bid bond required
4. Addenda:
 - a. Addendum 01 scheduled for August 26, 2026.
 - i. Contingency Allowance to be added.
 - ii. Geotechnical report.
 - iii. Additional framing details.
 - iv. Miscellaneous clarifications.
 - b. Additional addenda as required. Intent to issue nothing later than September 8, 2026.
5. Estimates:

a. General:	\$3,261,000
b. Mechanical:	\$1,046,250
c. <u>Electrical:</u>	<u>\$ 627,750</u>
d. Total:	\$4,935,000

B. General Requirements:

1. Type of Contracts: Multiple Prime for General, Mechanical, and Electrical. Option to submit a Single Prime bid.
2. Performance & payment bonds are required.
3. Project Coordinator: General Contractor.
4. Building Permits: By General Contractor
 - a. City of New Town already has the plans for a variance request.
5. Work by Others: Turf Field – A separate bid package & contract will be issued for a turf field.
 - a. Work to take place over the summer of 2027

**MEETING AGENDA** *continued*

6. Construction Schedule:
 - a. Start upon the award of the contract.
 - b. Substantially complete by August 13, 2027.
- C. Project Requirements/Highlights:
 1. Demolition of existing press box and seating.
 2. Wood framed building.
 - a. Glulam
 - b. Structural Insulated Panels (SIPs)
 3. Sloping site requires retaining walls.
 4. Grandstands are delegated design.
 - a. Railing by miscellaneous steel. Coordinate connection details.
 5. Relocation of two (2) field lighting poles.
 6. Site Usage
 - a. Limited staging area immediately around the site. Additional area may be available to the east of the track. Restore grass upon completion of the project.
 - b. Job trailers along on-street parking area.
 - c. Protect track and running surface.
- D. Questions
- E. Walkthrough of site

Next Meeting	Bid Opening, 2:00pm CDT, Thursday, September 10, 2026
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Reported By	Paul Breiner
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The preceding represents the understanding of the items discussed and decisions reached during and after this meeting. Please forward any comments/concerns/corrections/omissions to paul.breiner@eapc.net as soon as possible.

Report of Geotechnical Exploration Program

**Proposed Grandstands and Football Field
New Town, North Dakota
MTS #G26-043**

For

New Town School District

August 24, 2026



August 24, 2026

Attn: Mr. Marc Bluestone
New Town School District 1
300 Eagle Drive
New Town, ND 58763

ref: **Geotechnical Exploration Program
Proposed Grandstands and Football Field
New Town, North Dakota
Laboratory Number MTS #G26-034**

Mr. Marc Bluestone,

Enclosed is the geotechnical report for the proposed project. We are transmitting this report as an electronic file in pdf format. If you require a hard copy, please contact us. The work was conducted in general accordance with our proposal dated June 1, 2026.

Approximately 50 percent of the soil samples will be stored at the laboratory for a period of approximately 30 days from the date of this report. The samples will then be discarded unless we are requested to store them for a longer period.

We appreciate the opportunity to be of service to you on this project. If there are questions about the data or our recommendations, please contact us at 701-852-5553. Also, please contact us when you are ready for excavation observations and compaction test of controlled fill.

Sincerely,

MATERIAL TESTING SERVICES, LLC

A handwritten signature in black ink, appearing to read "Tanner Sanders", is written over a white rectangular background.

Tanner Sanders, PE
Geotechnical Engineer

MINOT
P.O. Box 634
Minot, ND 58702
701.852.5553



WILLISTON
7101 W 2nd Ave
Williston, ND 58801
701.572.4226

materialtestingservicesnd.com

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New Town, North Dakota
MTS #G26-034**

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APPENDICES

APPENDIX A – FIELD EXPLORATION PROGRAM

- A.1 Exploration Scope
- A.2 Surface Observations
- A.3 Subsurface Conditions
- A.4 Water Levels
- A.5 Soil Sampling
- A.6 Soil Classification Procedure

Attachments to Appendix A

Location Maps
Profile Log
Boring Log
Symbols & Descriptive Terminology on Test Boring Logs
Soil Classification Sheet

APPENDIX B – LABORATORY TEST PROGRAM

- B.1 Testing Scope
- B.2 Test Methods

Attachments to Appendix B

Mechanical Sieve Analysis
Unconfined Compression

APPENDIX C

Precautions for Excavating & Refilling During Cold Weather

Report of Geotechnical Exploration

**Proposed Grandstands and Football Field
New Town, North Dakota
MTS #G26-034**

1. INTRODUCTION

1.1. Authorization

The work was conducted in general accordance with our proposal dated June 1, 2026.

1.2. Scope of Services

The authorized scope of services included soil borings, laboratory testing and an engineering report.

Authorized drilling included four soil borings drilled to a nominal depth of 10 to 20 feet. Soil sampling was to be performed using standard penetration test (SPT) procedures. Laboratory tests in the proposal included moisture content, dry density, gradation through a #200 sieve, Atterberg limits (liquid & plastic limits) and unconfined compression testing. If soft cohesive soils were encountered undisturbed 3-inch diameter thin-walled Shelby tubes were to be taken.

The authorized engineering report includes the results of the field and laboratory testing as well as engineering recommendations regarding:

- a. Site preparation
- b. Foundation types and depths
- c. Allowable bearing capacity and estimated potential foundation settlement
- d. Potential construction difficulties
- e. Potential expansive or compressible soils
- f. Site drainage
- g. Exterior backfill
- h. Floor slabs
- i. Frost considerations
- j. Lateral Earth Pressures
- k. Excavation slopes
- l. Construction monitoring

Determining if there is potential on-site contamination is not included in the scope of services.

2. ENGINEERING REVIEW

2.1. Project Data

If the project information presented below is not correct or has been changed, it is necessary that the correct project data be presented to us for further review.

The proposed project will consist of construction of a new synthetic turf football field and associated grandstand facilities, including demolition of the existing grandstands. We anticipate that the proposed grandstands will be supported by strip footings with additional isolated column footings or piers. Portions of the proposed structure may be heated, while exterior foundations will be in unheated environments. The anticipated structural loads include isolated column loads of approximately 50 kips and strip footing loads of up to approximately 10 kips per linear foot.

2.2. Special Concerns and Constructability

Fill was encountered in the soil borings located in the proposed synthetic turf down to a depth of 5 feet below existing grade. It is our understanding that the existing fill will be left in place within the proposed turf fields.

Groundwater was encountered in SB-01 at a depth of approximately 12 feet below existing grade. Groundwater levels may fluctuate seasonally and in response to precipitation. Based on the observed groundwater levels, groundwater is not anticipated to significantly affect typical foundation excavations; however, localized seepage or perched water may be encountered.

We recommend bearing all foundations on native material or controlled compacted fill as described in Section 2.3. All construction and demolition debris should be removed from areas supporting foundations and structural slabs prior to construction. Existing fill containing construction debris or other unsuitable materials should be completely removed from foundation areas and replaced with approved controlled compacted fill.

2.3. Excavation and Site Preparation

We assume the site is currently near finished grade and that only minor additional earthwork will be required to achieve the proposed grades.

If excavations extend below the bottom of footing depth, we recommend providing a lateral excavation oversize of 1 foot for each 1 foot of fill required below the bottom of the new foundations, as applicable. The oversize should be measured from the bottom and outside edge of the foundations.

Loose lifts of new fill should not exceed 8 inches in thickness. Fill supporting footings or structural floor slabs should be compacted to at least 95 percent of the maximum dry density determined in accordance with ASTM D698 (Standard Proctor). The soils encountered in the vicinity of the proposed grandstands in soil borings SB-01 and SB-04 appear generally suitable for reuse as structural fill, subject to approval at the time of construction. Fat clay, if encountered during excavation, should not be reused beneath slabs or foundations and should instead be placed in landscaping areas. Imported fill beneath slabs may consist of pit-run sand or sand and gravel with a maximum particle size of 2 inches. Sand should be moisture conditioned as necessary to facilitate compaction, and lean clay should generally be placed within approximately -2 to +3 percent of its optimum moisture content as determined by ASTM D698.

We recommend that the excavated areas be observed by the geotechnical engineer of record or their representative prior to the placement of concrete or controlled, compacted fill. The purpose of the observations would be to make sure that no existing fill or soft natural soil is left in place and that the exposed natural soils can support the proposed fill and structural loads. We wish to point out that there could be deeper excavations required away from the boring locations.

We recommend that the topsoil and existing fill be removed from the building footprint prior to construction. A hydraulic excavator with a smooth cutting edge on the bucket should be

used for foundation excavation to minimize disturbance of the bearing soils. If the bottom of the excavation exposes native clay, the clay should **not** be surface compacted prior to placement of controlled, compacted fill. However, loose or disturbed materials should be removed to the extent practical. Where native sand is exposed at the bottom of the excavation, the sand should be surface compacted with a minimum of three passes of a heavy vibratory roller prior to placement of controlled, compacted fill.

After the excavations are complete, backfill should be placed as soon as possible. Also, care should be taken so that the grades slope away from the excavations in the event it rains. Although it is impossible to completely keep rainwater from the site, provisions must be made to protect the excavations as much as possible from the influx of rainwater or surface runoff both during and after construction. Water and any soft/saturated soil should be removed as soon as possible.

If earthwork is done during periods of freezing temperatures, we recommend protecting the fill from freezing once it has been placed. No frozen soils should be used as fill and fill should not be placed on frozen ground. Earthwork could be difficult in the spring or late fall when conditions are often cool and wet.

If groundwater is encountered at the bottom of foundation excavations, we recommend contacting us for additional recommendations.

2.4. Isolated Piers

It was indicated to us that there will be some exterior isolated piers supported on footing pads. We anticipate that isolated columns will be bear on native soil or controlled compacted fill. A design frost depth of 7 feet below final grade should be used for footings or foundations located in unheated environments. Site preparation beneath isolated foundations should follow the recommendations provided in Section 2.3.

Groundwater was observed at approximately 12 feet below existing grade in SB-01. Groundwater is not anticipated to significantly affect typical foundation excavations; however, localized groundwater seepage or perched water may be encountered. If groundwater or saturated soils are encountered at the proposed footing elevation, we should be contacted for additional recommendations. Depending on the conditions encountered, over-excavation and replacement with controlled density fill (CDF) or other approved structural fill may be required.

2.5. Retaining Walls

We recommend using a design frost depth of 7 feet below final grade for retaining walls in unheated environments. Site preparation should follow recommendations set forth in Section 2.3.

Retaining walls should be constructed with Class 5 aggregate backfill meeting the requirements described below. To reduce lateral earth pressures relative to those associated with the on-site cohesive soils, Class 5 aggregate containing less than 7 percent material passing the No. 200 sieve should be placed within the zone immediately behind the wall. The select backfill should extend from the base of the wall upward and outward beyond the edge of the footing at a slope of 1H:1V or flatter. With the specified Class 5 backfill zone, the retaining wall should be designed for an at-rest equivalent fluid pressure of 55 pcf and an active equivalent fluid pressure of 35 pcf.

For lateral resistance, an ultimate equivalent fluid pressure of 330 pcf may be used for undisturbed native clay or silty clay and 500 pcf for properly compacted Class 5 aggregate. Material placed on the passive side of the wall should be compacted to at least 95 percent of the maximum dry density determined in accordance with ASTM D698. Portions of foundations within 5 feet of final grade in unheated areas should be neglected when calculating passive resistance because of potential frost softening. An ultimate friction coefficient of 0.30 may be used between the bottom of the footing and the supporting foundation soils. These values represent ultimate resistance, and a theoretical factor of safety

of 2.0 should be applied for design.

The weight of the concrete foundation system and the weight of properly compacted soil placed over the foundation may be considered when evaluating resistance to overturning and uplift. A total unit weight of 145 pcf may be used for concrete and 100 pcf for compacted soil. Fill relied upon for uplift resistance should be compacted to at least 95 percent of the maximum dry density determined in accordance with ASTM D698. A theoretical factor of safety of 2.0 or greater should be used when evaluating uplift resistance.

Weep holes or drain tile should be provided that hydrostatic forces do not build up behind the wall in the event of leaking utilities or the influx of surface water. Additionally, we recommend placing clay cap at the surface on top of the wall to help divert water away from the retaining wall.

2.6. Synthetic Turf Field Subgrade

It is our understanding that the aggregate base and drainage courses will be designed by others.

The existing soils are frost susceptible. Therefore, seasonal frost penetration may result in frost heave and differential movement beneath the proposed synthetic turf system. Variations in fill thickness and composition may result in localized differential frost movement. The recommended subgrade preparation, compaction, and drainage measures are intended to reduce the potential for frost-related movement; however, some seasonal movement should be anticipated.

We recommend stripping any vegetation and topsoil prior to construction. The subgrade can then be excavated to the proper elevation for the placement of the drainage and base course. The subgrade should be properly graded to aid in removing any surface water from the field.

The exposed subgrade should be scarified to a minimum depth of 12 inches, moisture conditioned and recompact to at least 98 percent of the maximum dry density determined in accordance with ASTM D698. The moisture content should generally be maintained within -4 to +3 percent of the optimum moisture content.

Based on the observed field conditions, the existing subgrade soils below the football field are anticipated to be substantially drier than optimum moisture content. Adequate moisture conditioning, including wetting, will likely be required to achieve the recommended moisture content range prior to compaction.

The compacted subgrade should be proof rolled using a loaded end dump or water truck. Any soft spots detected should be stabilized prior to placing any aggregate course on the subgrade.

2.7. Floor Slabs

We recommend that the site be prepared in accordance with the recommendations provided in Section 2.3. Slabs-on-grade should be constructed independently of foundation walls and columns to minimize the potential for differential movement between the slab and structural foundations. A minimum 6-inch layer of free-draining sand should be provided beneath slabs. The sand should contain less than 7 percent material passing the No. 200 sieve by weight and, where gravel is present, the maximum particle size should be limited to 1 inch. The granular layer will provide a suitable working surface for concrete placement and reduce the potential for capillary moisture migration into the slab.

With the recommended site preparation, a modulus of subgrade reaction of 200 pci may be used for slab design where a minimum of 18 inches of properly compacted granular fill is provided beneath the slab. Where clay fill extends to within 6 inches of the bottom of the slab, a modulus of subgrade reaction of 150 pci may be used. These values assume the supporting fill is properly placed and compacted in accordance with Section 2.3.

2.8. Frost Considerations

Footings should be placed at the frost depth indicated in Section 2.9 unless an alternative frost-protection system is incorporated into the design. No frozen soil should be used as fill, and fill should not be placed on frozen ground. Fill placed and compacted during cold weather should be protected from freezing until construction is complete and the structure can provide adequate protection from frost. Additional precautions for cold-weather excavation and placement of fill are provided in Appendix C.

2.9. Foundations Recommendations**2.9.1 Footing Depths**

Perimeter footings for heated buildings should be carried to a frost depth of 5 feet. The frost depth should be considered from final grade to the bottom of the footing. Interior footings can be placed at a convenient depth below the floor slab. For footings which will be in an unheated environment, we recommend using a frost depth of 7 feet.

2.9.2 Bearing Capacity and Settlement

With the recommended site preparation, on-site observations by the geotechnical engineer, and adequate compaction testing of controlled compacted fill, columns and wall foundations may be proportioned for a net allowable soil bearing pressure of up to 2,000 psf. This allowable bearing pressure is based on a theoretical factor of safety of approximately 3 or more with respect to general shear failure of the supporting soils.

Our calculations suggest that total settlements should be 1 inch or less and that the differential settlement should be ½ inch or less.

2.9.3 Lateral Earth Pressures

The following lateral earth pressure recommendations apply to general foundation walls and do not apply to the retaining wall recommendations provided in Section 2.5. The values assume that the backfill is properly compacted and that adequate drainage is provided behind the wall. Walls that are restrained against movement should be designed for at-rest lateral

earth pressures. For on-site lean clay, an equivalent fluid pressure of 65 pcf may be used for at-rest conditions above the groundwater level. For imported compacted sand or sand with gravel, an equivalent fluid pressure of 55 pcf may be used for at-rest conditions.

Where sufficient wall movement can occur to mobilize active conditions, an equivalent fluid pressure of 45 pcf may be used for compacted on-site clay and 40 pcf for compacted imported sand or sand with gravel above the groundwater level. For passive resistance, an ultimate equivalent fluid pressure of 330 pcf may be used for on-site clay and 425 pcf for compacted imported sand or sand with gravel. If compacted fill is relied upon for passive resistance, it should be compacted to at least 95 percent of the maximum dry density determined in accordance with ASTM D698. Portions of foundations within 5 feet of final grade in unheated areas should be neglected when calculating passive resistance due to frost softening. An ultimate friction coefficient of 0.30 may be used between the bottom of the footing and the supporting foundation soils.

We wish to point out that these values will give the ultimate resistance to lateral loads. We recommend that a theoretical safety factor of 2.0 be applied for a safe design.

2.9.4 Methods of Analysis

The allowable foundation loading recommended was arrived at using the Terzaghi-Meyerhof bearing capacity equation with estimates using empirical correlations with the "N" values, as well as our experience with similar site and soil conditions.

Settlement estimates were based on empirical correlations between SPT "N" values and pressuremeter modulus, with consideration given to soil type. The estimated pressuremeter modulus was then used in the pressuremeter method of analysis.

The equivalent passive, at-rest and active earth pressures were calculated using estimated unit weights and estimated angles of internal friction based on our experience, the laboratory testing and correlations with the Atterberg limits and soil type.

2.10 Exterior Foundation Backfill

The exterior foundation backfill above the footings could consist of the on-site non-organic sandy lean clay fill, or off-site lean clay or pit-run sand and gravel. The exterior backfill should be placed in loose lift thicknesses not to exceed 8 inches. Compaction should be to a minimum of 92 percent of the standard Proctor density in lawn areas. For sidewalks or lightly loaded structures such as air conditioning units, the compaction should be increased to 95 percent. If there will be driveways or parking areas within the backfill zone the fill 1 foot or more below the bottom of pavements should be compacted to at least 95 percent and fill within 1 foot should be compacted to at least 98 percent. Clay fill should be placed at a moisture content of -3 to +3 percent of the optimum moisture content and sand fill should be moisture conditioned as required to facilitate compaction.

2.11 Exterior Slabs

Due to the potential for frost movement, precautions should be taken to minimize post-construction movement of sensitive exterior slabs. For slabs that are sensitive to differential frost movement, one option is to excavate frost-susceptible soils to a depth of approximately 5 feet below the slab and replace them with non-frost-susceptible sand containing less than 5 percent material passing the No. 200 sieve. An alternative is to provide a minimum of 4 inches of extruded polystyrene insulation beneath the slab and extend the insulation at least 8 feet laterally beyond the slab perimeter, with approximately 6 to 12 inches of protective sand placed above the insulation. A third alternative is to support the slab or steps on foundations extending to the applicable frost depth. Where this foundation-supported option is used, a minimum 4-inch void should be provided below the slab to accommodate potential frost movement.

Sensitive slabs include slabs that cannot tolerate significant differential movement without causing functional or structural problems, such as sidewalks or steps adjacent to building entrances. This recommendation is not intended to imply that all exterior sidewalks, driveways, or slabs require these measures.

2.12 Excavation Slopes

Excavations should be maintained in accordance with applicable OSHA requirements and the soil conditions encountered at the time of construction. The excavation contractor is responsible for maintaining safe excavation conditions, and the contractor's competent person should evaluate the soil type and required protective system in accordance with current OSHA requirements. Excavation slopes and protective systems should be adjusted as necessary based on actual field conditions, groundwater, excavation depth, and the presence of adjacent structures or utilities. MTS does not assume responsibility for site safety or the activities of the excavation contractor.

2.13 Surface Drainage

We advise that adequate drainage be maintained during and after construction. Unless roof drainage is internal, gutters and downspouts or an appropriate closed-conduit drainage system should be provided to control roof runoff. Downspouts should discharge through extensions that carry roof water well beyond the building backfill zone, and splash pads should be provided where appropriate.

Where sand is used as exterior foundation backfill, a minimum 2-foot-thick clay cap should be provided at the ground surface in landscaped areas to reduce infiltration and help divert surface water away from the building.

Exterior clay backfill should slope away from the building at a rate of at least 1 inch per foot for a minimum distance of 10 feet in lawn areas. In parking and paved areas, positive surface drainage should be provided to the extent practical.

3 CONSTRUCTION OBSERVATION AND TESTING

The recommendations contained in this report have been made based on the subsurface conditions found at the boring locations. It is possible that there are soil conditions on site that were not represented by the borings. Also, we presumed that close construction

monitoring during excavation and backfilling would be authorized. Consequently, on-site observation during construction is considered integral to the successful implementation of the recommendations.

It is imperative that the geotechnical engineer be on site at the following times to observe the site conditions and effectiveness of the construction. We recommend that the testing be performed by the geotechnical engineer as the **Owner's** representative during construction.

3.1 Excavation Observations

The geotechnical engineer should observe the entire excavation bottom prior to placement of engineered fill and/or concrete. The geotechnical engineer would also be available for additional consultation and recommendations if necessary.

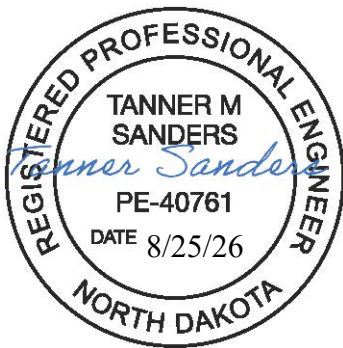
3.2 Placement of Fill

It will also be necessary to perform a representative number of compaction tests during placement of engineered fill. The tests should be performed to determine if the required compaction was achieved. As a general guideline, tests should be taken for each 2,500 square feet embankment fill for structures, every 5000 square feet for parking areas, every 75 to 100 feet in trench fill, and for each 2-foot thickness of fill. The actual number of tests should be left to the discretion of the geotechnical engineer.

4 EXPLORATION LIMITATIONS

The recommendations contained in this report represent our professional opinions. These opinions were arrived at according to currently accepted engineering practices at this time and location. Other than this, no warranty is intended or implied.

This report is written by:



Tanner Sanders, PE
Geotechnical Engineer

Reviewed by:

A handwritten signature in black ink that reads "Steve Wald".

Steve Wald, PE
President

APPENDIX A – FIELD EXPLORATION PROGRAM

- A.1 Exploration Scope
- A.2 Surface Observations
- A.3 Subsurface Conditions
- A.4 Water Levels
- A.5 Soil Sampling
- A.6 Soil Classification Procedure

Attachments to Appendix A

Location Maps

Boring Log

Symbols & Descriptive Terminology on Test Boring Logs

Soil Classification Sheet

A. FIELD EXPLORATION PROGRAM

A.1 Exploration Scope

Four borings were drilled to nominal depths of 10 to 20 feet for the project on July 16, 2026. The boring location is illustrated on the attached drawing at the back of this Appendix.

The boring was backfilled with on-site materials and some settlement of these materials can be expected to occur. The final closure of the hole is the responsibility of the client or property owner.

A.2 Surface Observations

The site is located at 206 2nd Avenue E in New Town, North Dakota and is currently used as a football field with grandstands or bleachers.

The property is fairly flat, with boring elevations ranging from 96.1 feet to 96.7 feet assumed datum. The TBM (temporary benchmark, elevation 100.0') was the top of the retaining wall at the west corner of existing grandstands.

A.3 Subsurface Conditions

The subsurface conditions encountered at the test location are illustrated by means of the attached boring log. We wish to point out that the subsurface conditions at other times and locations at the site may differ from those found at our test boring locations. If different conditions are encountered during construction, it is necessary that you contact us so that our recommendations can be reviewed. The test boring log also shows the possible geologic origin of the materials encountered.

Fill or possible fill were encountered at SB-02 and SB-03 at depth of approximately 5 feet below existing grade. Borings SB-01 and SB-04 encountered up to 6 inches of topsoil. Below the topsoil and fill, naturally deposited silty sand, sand with silt, lean clay and fat clay were encountered to the termination depth.

Based on the standard penetration resistance ("N" values), the clayey soils were typically soft to firm in consistency. The sands were typically very loose to medium dense.

A.4 Water Levels

Groundwater measurements were made in the test boring during drilling and at completion of drilling before the boring was backfilled. This information is shown at bottom of the attached boring log.

Measurable groundwater was observed at approximately 12.0 feet below existing grade.

Water levels should be expected to fluctuate seasonally and annually. The water levels at the time of construction could be significantly different than what was recorded on the boring logs. The time of year the borings were drilled and the history of precipitation prior to drilling should be known when using the groundwater readings on the boring logs to extrapolate water levels at other points in time.

A.5 Soil Sampling

Soil sampling was done according to the procedures described by ASTM D1586. Using this procedure, a 2-inch O.D. split barrel sampler is driven into the soil by a 140-lb weight falling 30 inches. After an initial set of 6 inches, the number of blows required to drive the sampler an additional 12 inches is known as penetration resistance or "N" value. The "N" value is an index of the relative density of cohesionless soils and the consistency of cohesive soils.

We are retaining representative samples of the soil obtained during our field operations for approximately one month. We will then discard them unless we are notified further as to their disposition.

A.6 Soil Classification Procedure

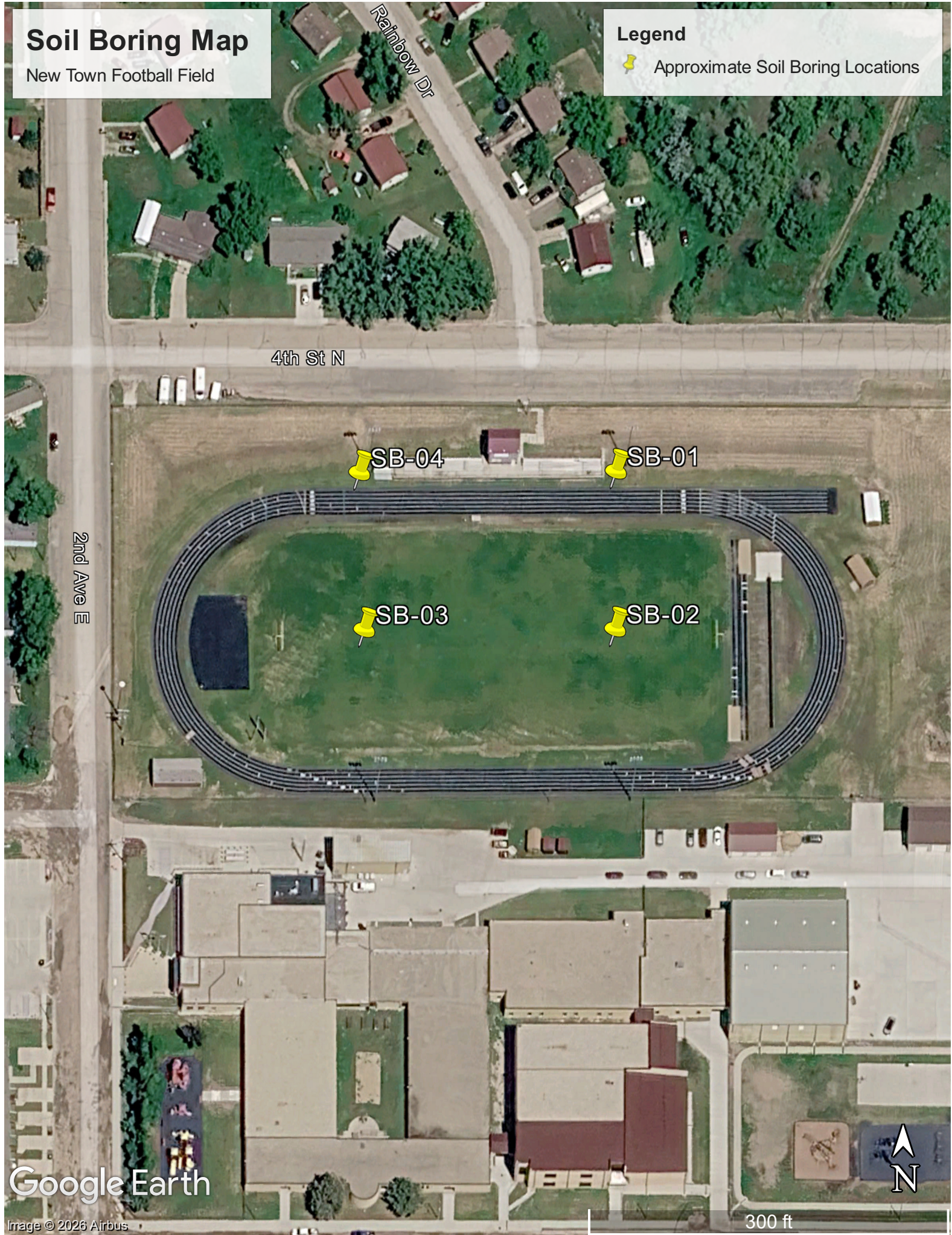
As the samples were obtained in the field they were visually and manually classified by the crew chief according to ASTM D 2488. Representative portions of all samples were then sealed and returned to the laboratory for further examination and for verification of the field classification. In addition, selected samples were then submitted to a program of laboratory tests. Logs of the borings indicating the depth and identification of the various strata, the "N" value, the laboratory test data, water level information and pertinent information regarding the method of maintaining and advancing the drill holes are also attached. Charts illustrating the soil classification procedures, the descriptive terminology and symbols used on the boring logs are also attached.

Soil Boring Map

New Town Football Field

Legend

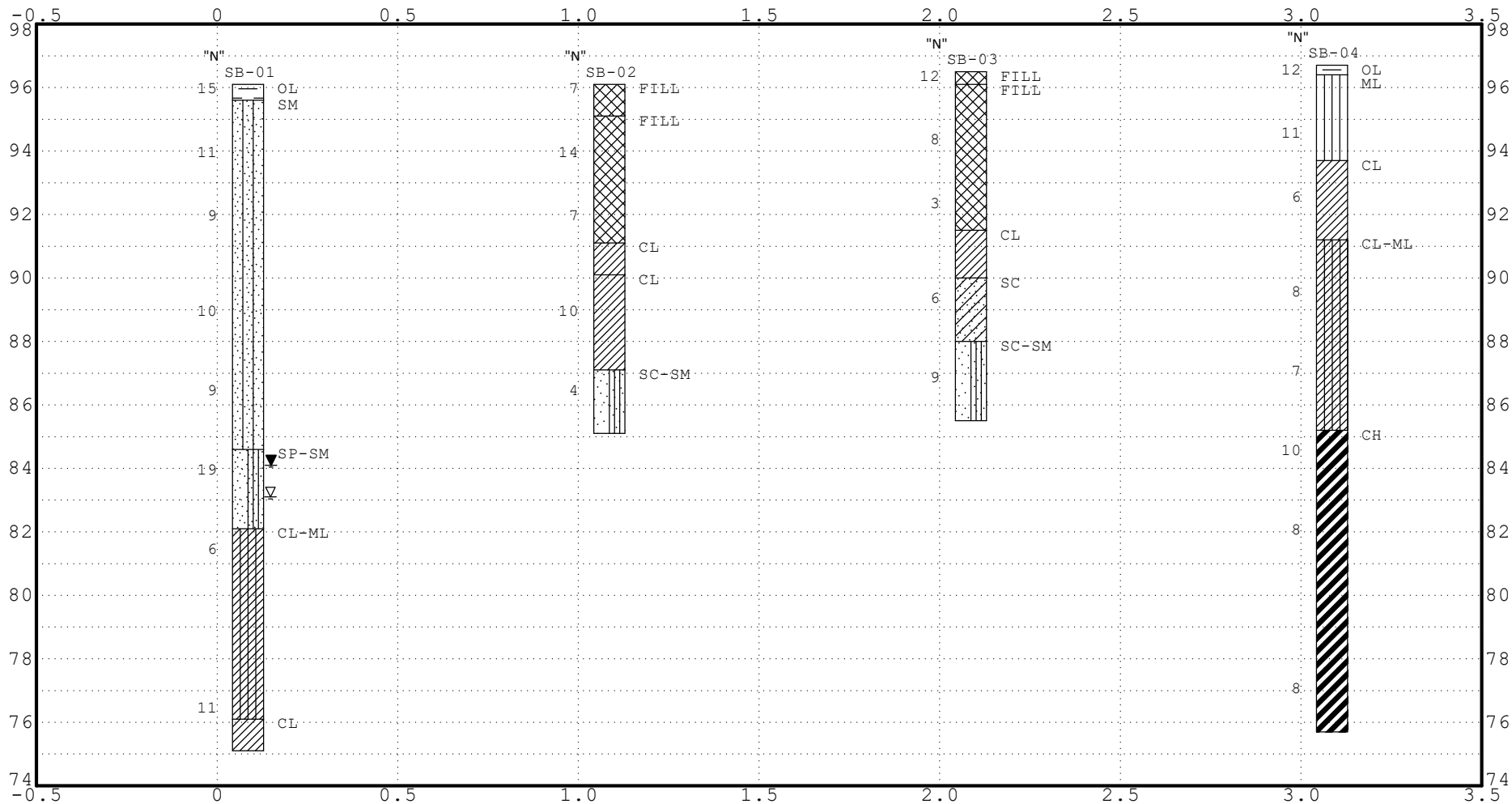
 Approximate Soil Boring Locations



Google Earth

Image © 2026 Airbus

300 ft



Boring	North	East	Elev.	Depth
SB-01	0	1	96.1	21.0
SB-02	0	2	96.1	11.0
SB-03	0	3	96.5	11.0
SB-04	0	4	96.7	21.0

SOIL PROFILE DIAGRAM		
Material Testing Services Inc		
Grand Stands		
New Town, ND		
PROJECT #	DATE	PLATE
G26-034	Aug 26	1

MATERIAL TESTING SERVICES, LLC

Box 634

Minot, North Dakota 58702

(701) 852-5553

SOIL BORING RECORD

BORING NUMBER

SB-01

SHEET 1 OF 1

PROJECT

Grand Stands

PROJECT LOCATION

New Town, ND

PROJECT NUMBER

G26-034

START DATE

7/16/26

FINISH DATE

7/16/26

LAYER DEPTH/ ELEVATION (FT)	SOIL DESCRIPTION	SYMBOLIC LOG	ELEVATION/ DEPTH (FT)	GEOLOGY	SAMPLE					TEST RESULTS				
					TYPE	LEGEND	D (PCF)	N VALUES	BLOWS/FT	WATER LEVEL	MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	Qu (ksf)
0.5	Topsoil / Organic Lean Clay , dark brown, moist (OL)		96.1	Topsoil	SB			15						
95.6	Silty Sand , light brown, moist, loose to medium dense (SM)			Mixed Alluvium	SB			11			10			
					SB			9			10			
			91.5		SB									
					SB			10			13			
					SB			9						
11.5	Sand with Silt , brown, wet, fine to coarse grained, medium dense (SP-SM)		86.1		SB			19						
84.6					SB									
14.0	Silty Clay , brown, frequent silt lenses, moist to wet, soft to firm (CL-ML)		82.1		SB			6			26			
82.1			81.1											
														
20.0					SB			11						
76.1	Lean Clay , brown, iron oxide staining, moist to wet, firm (CL)		76.1											
21.0														
75.1	End of Boring NM = None Measured													

DRILLER

RB

METHOD

4" HSA, 0-19.5'

LOGGER

JF

REVIEWER

TS

DRILL RIG

CME 45

WATER LEVEL
MEASUREMENTS

DATE

TIME

SAMPLED
DEPTH

CASING
DEPTH

CAVE-IN
DEPTH

DRILLING
MUD LEVEL

WATER
LEVEL

7/16/26

1055

14

13

7/16/26

1300

13

12

MATERIAL TESTING SERVICES, LLC

Box 634

Minot, North Dakota 58702

(701) 852-5553

SOIL BORING RECORD

BORING NUMBER

SB-02

SHEET 1 OF 1

PROJECT

Grand Stands

PROJECT LOCATION

New Town, ND

PROJECT NUMBER

G26-034

START DATE

7/16/26

FINISH DATE

7/16/26

LAYER DEPTH/ ELEVATION (FT)	SOIL DESCRIPTION	SYMBOLIC LOG	ELEVATION/ DEPTH (FT)	GEOLOGY	SAMPLE					TEST RESULTS			
					TYPE	LEGEND	D (PCF)	N VALUES	BLOWS/FT	WATER LEVEL	MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC LIMIT (%)
1.0	<u>Fill Topsoil / Organic Lean Clay</u> , dark brown, moist (OL)		96.1	Fill	SB			7			7		
95.1	<u>Fill / Sandy Lean Clay</u> , dark brown to brown mixed, moist (CL)				SB			14					
5.0					SB			7					
91.1	<u>Sandy Lean Clay</u> , dark brown, moist, soft (CL)		91.1	Mixed Alluvium									
6.0													
90.1	<u>Lean Clay</u> , light brown, moist, firm (CL)				SB			10					
9.0													
87.1	<u>Silty Clayey Sand</u> , light brown, moist, fine grained, very loose (SC-SM)		86.1		SB			4					
11.0													
85.1	End of Boring NM = None Measured												

DRILLER **RB**
METHOD **4" HSA, 0-9.5'**
LOGGER **JF**
REVIEWER **TS**
DRILL RIG **CME 45**

WATER LEVEL
MEASUREMENTS

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING MUD LEVEL	WATER LEVEL
7/16/26	1125			11.5		NM

MATERIAL TESTING SERVICES, LLC

Box 634

Minot, North Dakota 58702

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SOIL BORING RECORD

BORING NUMBER

SB-03

SHEET **1** OF **1**

PROJECT

Grand Stands

PROJECT LOCATION

New Town, ND

PROJECT NUMBER

G26-034

START DATE

7/16/26

FINISH DATE

7/16/26

LAYER DEPTH/ ELEVATION (FT)	SOIL DESCRIPTION	SYMBOLIC LOG	ELEVATION/ DEPTH (FT)	GEOLOGY	SAMPLE					TEST RESULTS				
					TYPE	LEGEND	D (PCF)	N VALUES	BLOWS/FT	WATER LEVEL	MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	Qu (ksf)
96.4	Fill Topsoil / Organic Lean Clay , dark brown, moist (OL)		96.5	Fill	SB			12						
96.1	Fill / Sandy Lean Clay , dark brown to brown mixed, moist (CL)				SB			8		9				
5.0					SB			3						
91.5	Sandy Lean Clay , dark brown, moist, very soft (CL)			91.5	Mixed Alluvium									
6.5														
90.0	Clavey Sand , light brown, moist, loose (SC)				SB		6							
8.5														
88.0	Silty Clavey Sand , light brown, moist, fine grained, loose (SC-SM)													
86.5				86.5		SB		9						
11.0														
85.5	End of Boring NM = None Measured													
DRILLER METHOD LOGGER REVIEWER DRILL RIG RB 4" HSA, 0-9.5' JF TS CME 45				WATER LEVEL MEASUREMENTS	DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING MUD LEVEL	WATER LEVEL			
					7/16/26	1150			11.0		NM			

MATERIAL TESTING SERVICES, LLC

Box 634

Minot, North Dakota 58702

(701) 852-5553

SOIL BORING RECORD

BORING NUMBER

SB-04

SHEET 1 OF 1

PROJECT

Grand Stands

PROJECT LOCATION

New Town, ND

PROJECT NUMBER

G26-034

START DATE

7/16/26

FINISH DATE

7/16/26

LAYER DEPTH/ ELEVATION (FT)	SOIL DESCRIPTION	SYMBOLIC LOG	ELEVATION/ DEPTH (FT)	GEOLOGY	SAMPLE					TEST RESULTS				
					TYPE	LEGEND	D (PCF)	N VALUES	BLOWS/FT	WATER LEVEL	MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	Qu (ksf)
96.3	Topsoil / Organic Lean Clay , dark brown, moist (OL)		96.7	Topsoil	SB			12						
96.4	Silt with Sand , light brown, moist, firm (ML)			Mixed Alluvium	SB			11						
3.0					SB			6		20				
93.7	Sandy Lean Clay , light brown, frequent silt lenses, moist, soft to firm (CL)				SB			8		24	28	21		
5.5				91.7				7						
91.2	Silty Clay , light brown, moist, fine grained, soft (CL-ML)				SB			10		36	52	21		
				86.7		SB		8						
11.5														
85.2	Fat Clay , grayish brown, iron oxide staining, moist to wet, soft to firm (CH)				SB									
					SB									
			81.7											
			76.7		SB			8						
21.0														
75.7	End of Boring NM = None Measured													

DRILLER **RB**
METHOD **4" HSA, 0-19.5'**
LOGGER **JF**
REVIEWER **TS**
DRILL RIG **CME 45**

WATER LEVEL
MEASUREMENTS

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	DRILLING MUD LEVEL	WATER LEVEL
7/16/26	1255			18.5		NM

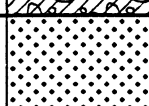
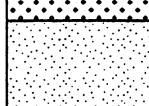
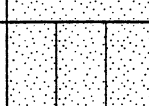
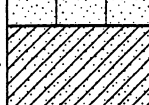
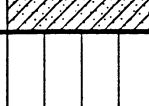
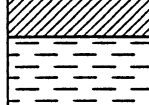
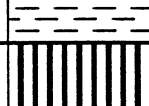
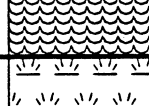
SYMBOLS AND DESCRIPTIVE TERMINOLOGY ON TEST BORING LOG

<u>Symbols for Drilling and Sampling</u>		<u>Symbols for Laboratory Tests</u>	
<u>Symbol</u>	<u>Description</u>	<u>Symbol</u>	<u>Description</u>
HSA	3 1/4" I.D. hollow stem auger	W	Water content
_FA	4", 6" or 10" diameter flight auger	D	Dry density - pounds per cubic foot
_HA	2", 4" or 6" hand auger	LL	Liquid limit - ASTM** D 4318
_DC	2 1/2", 4", 5" or 6" steel drive casing	PL	Plastic limit - ASTM D 4318
_RC	Size A, B or N rotary casing		
PD	Pipe drill or cleanout tube		--- Inserts in Last Column (Qu or RQD) ---
CS	Continuous split barrel sampling	Qu	Unconfined compressive strength, psf - ASTM D 2166
DM	Drilling mud	Pq	Penetrometer reading, tsf
JW	Jetting water	Ts	Torvane reading, tsf
SB	2" O.D. split barrel sampling	G	Specific gravity
_L	2 1/2" or 3 1/2" O.D. SB liner sample	SL	Shrinkage limits - ASTM D 427
_T	2" or 3" thin walled tube sample	OC	Organic content - Combustion method
3TP	3" thin walled tube using pitcher sampler	SP	Swell pressure, tsf
_TO	2" or 3" thin walled tube using Osterberg sampler	PS	Percent swell under pressure
W	Wash sample	FS	Free swell, percent
B	Bag sample	SS	Shrink swell, percent
P	Test pit sample	pH	Hydrogen ion content - Meter Method
_Q	BQ, NQ, or PQ wireline system	SC	Sulfate content, parts/million or mg/l
_X	AX, BX, or NX double tube barrel	CC	Chloride content, parts/million or mg/l
N	Standard penetration test, blows per foot	C*	One dimensional consolidation - ASTM D 2435
CR	Core recovery, percent	Qc*	Triaxial compression
WL	Water level	D.S.*	Direct shear - ASTM D 3080
?	Water level	K*	Coefficient of permeability, cm/sec
NMR	No measurement recorded, primarily due to presence of drilling or coring fluid	DH*	Double hydrometer - ASTM D 4221
		MA*	Particle size analysis - ASTM D 422
		R	Laboratory electrical resistivity, ohm-cm - ASTM G 57
		E*	Pressuremeter deformation modulus, tsf
		PM*	Pressuremeter test
		VS*	Field vane shear - ASTM D 2573
		IR*	Infiltrometer test - ASTM D 3385
		RQD	Rock quality designation, percent
		*	Results shown on attached data sheet or graph
		**	ASTM designates American Society for Testing and Materials

DESCRIPTIONS OF N-VALUES VS. SOIL PROPERTIES				DESCRIPTIONS OF SOIL CONDITIONS	
<u>N Value</u>	<u>Density</u>	<u>N Value</u>	<u>Consistency</u>	<u>Condition</u>	<u>Description</u>
0 - 4	Very loose	0 - 4	Very soft	Lamination	Up to 1/2" thick stratum
5 - 10	Loose	5 - 8	Soft	Layer	1/2" to 6" thick stratum
11 - 30	Medium dense	9 - 15	Firm	Dry	Powdery, no noticeable water
31 - 50	Dense	16 - 30	Hard	Moist	Below saturation
Over 50	Very dense	Over 30	Very hard	Wet	Saturated, above liquid limit
				Waterbearing	Pervious soil below water
				Varved	Alternating laminations of any combinations of clay, silt and fine grained sand

DESCRIPTIONS OF GRAVEL PROPORTIONS IN SOILS			DESCRIPTIONS OF PARTICLE SIZES	
Soil Type	Description	Range, %	Material Type	Size
Coarse grained soils	A little gravel	2 - 14	Boulders	Over 12"
Coarse grained soils	With gravel	15 - 49	Cobbles	3" - 12"
			Coarse gravel	3/4" - 3"
Fine grained soils:			Fine gravel	#4 sieve - 3/4"
71-85% passing #200 sieve	A little gravel	2 - 7	Coarse sand	#4 - #10 sieve
71-85% passing #200 sieve	With gravel	8 - 29	Medium sand	#10 - #40 sieve
70% passing #200 sieve	A little gravel	2 - 14	Fine sand	#40 - #200 sieve
70% passing #200 sieve	With gravel	15 - 24	Silt	100% passing #200 sieve and > 0.002mm
70% passing #200 sieve	Gravelly	16 - 49	Clay	100% passing #200 sieve and < 0.002mm

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	CLEAN GRAVELS (LITTLE OR NO FINES)		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
				GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
				GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
	SAND AND SANDY SOILS	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		SM	SILTY SANDS, SAND - SILT MIXTURES
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
			HIGHLY ORGANIC SOILS		

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

APPENDIX B – LABORATORY TEST PROGRAM

- B.1 Testing Scope
- B.2 Test Methods

Attachments to Appendix B

Mechanical Sieve Analysis
Unconfined Compression

B. LABORATORY TEST RESULTS

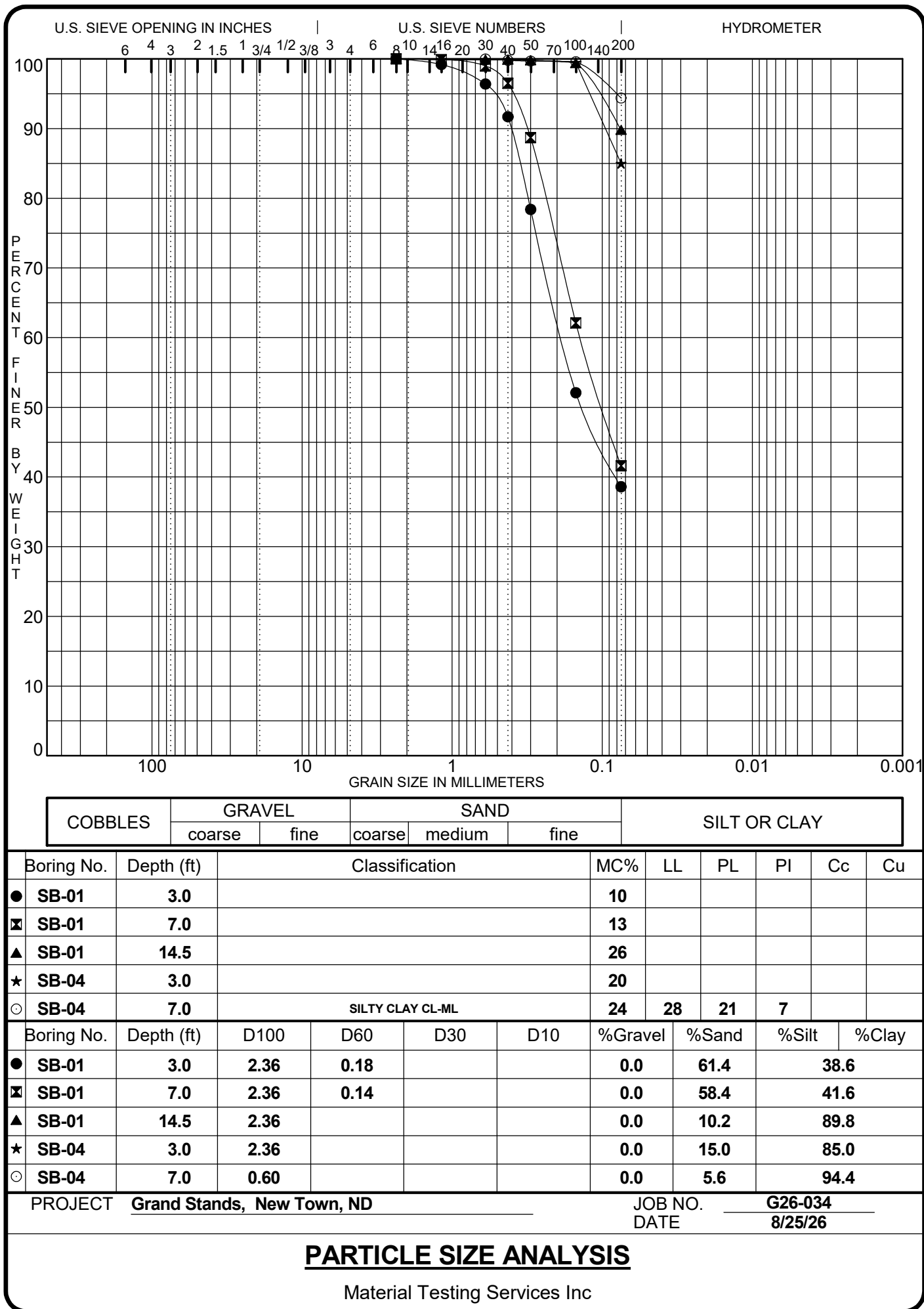
B.1. Testing Scope

Laboratory testing was conducted to characterize soil properties including mechanical sieve analysis, Atterberg limits, moisture contents, in-situ density, and unconfined compression. Results are listed on the boring logs or attached test reports.

B.2. Test Methods

Testing and classification of soils was performed in accordance with the following standards as applicable:

Description	ASTM Method
Unified soil classification system	D 2487
Atterberg limits	D 4318
In-situ moisture content	D 2216
Sieve analysis	D 422, C 117, C 136
Moisture-density relationship	D 698 or D 1557
In-situ dry density	D 7263
Unconfined compression, Qu	D 2166
CBR	D 1883



MATERIAL TESTING SERVICES, LLC

PO Box 634
Minot, ND 58702
(701) 852-5553

UNCONFINED COMPRESSIVE STRENGTH ASTM D 2166

7101 W 2nd Ave
Williston, ND 58801
(701) 572-4226

PROJECT: New Town Football Field
New Town, ND

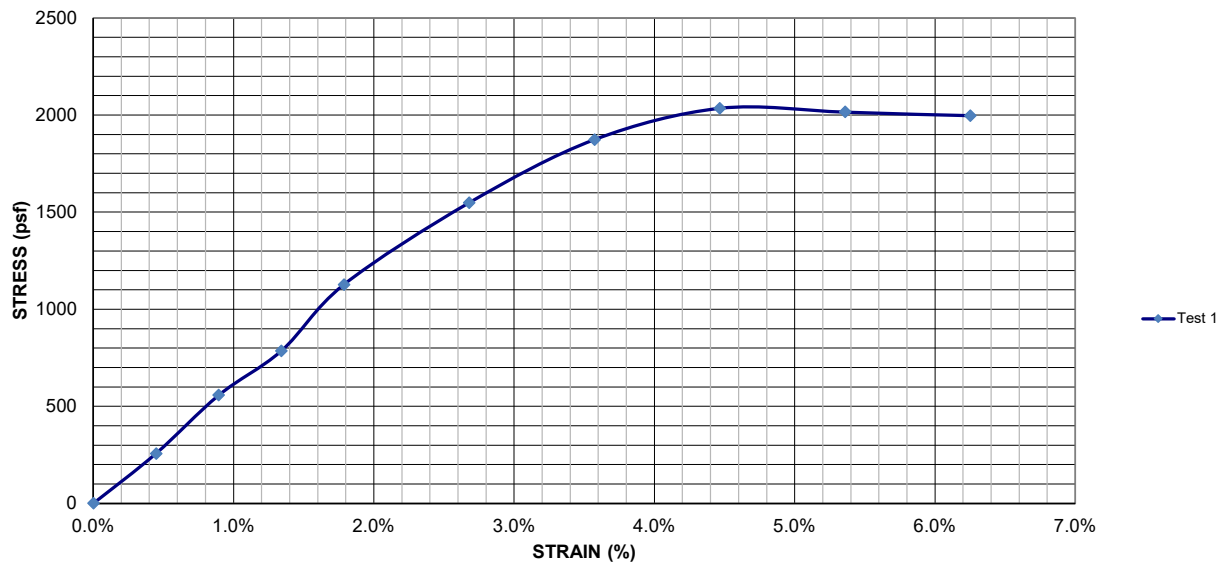
DATE: 11-Aug-26

COPIES TO:

REPORTED TO:

MTS No. G26-034

Specimen ID: Test 1
Boring 4
TW, 7-9 feet
Soil Class: Lean Clay (CL)
Dry Density (pcf): 103.4
Water Content: 11.9%
Sample Dia. (mm): 71.1
Sample Ht (mm): 142.2
Height/Diameter: 2.00
Unc. Strength (psf): 2034
Strain at Failure (%): 4.5



AS A MUTUAL PROTECTION TO CLIENTS, THE PUBLIC AND OURSELVES, ALL REPORTS ARE SUBMITTED AS THE CONFIDENTIAL PROPERTY OF CLIENTS, AND AUTHORIZATION FOR PUBLICATION OF STATEMENTS, CONCLUSIONS OR EXTRACTS FROM OR REGARDING OUR REPORTS IS RESERVED PENDING OUR WRITTEN APPROVAL.

Material Testing Services, LLC

by *SK Wall*

APPENDIX C

Precautions for Excavating & Refilling During Cold Weather

PRECAUTIONS FOR EXCAVATING AND REFILLING DURING COLD WEATHER

The winter in North Dakota presents specific problems for foundation construction. Soils which are allowed to freeze undergo a moisture volume expansion, resulting in a loss of density. These frost-expanded soils will consolidate upon thawing, causing settlement of any structure supported on them. To prevent this settlement, frost should not be allowed to penetrate below any proposed structure.

Ideally, winter excavation should be limited to areas small enough to be refilled to a grade higher than footing grade on the same day. Typically, these areas should be filled to floor grade. Trenching back down to unfrozen soils for foundation construction can then be performed just prior to footing placement. The excavated trenches should be protected from freezing by means of insulating or heating during foundation construction. Backfilling of the foundation trenches should be performed immediately after the below-grade foundation construction is finished. In addition, any interior footings, or footings designed without frost protection should be extended below frost depth, unless adequate precautions are taken to prevent frost intrusion until the building can be enclosed and heated.

In many cases, final grade cannot be attained in one day's time, even though small areas are worked. In the event final grade cannot be attained in one day's time, frost can be expected to develop overnight. The depth of frost penetration can be minimized by leaving a layer of loose soil on top of the compacted material overnight. However, any frost which forms in this loose layer, or snow, should never be used as fill material.

After the structure has been enclosed, all floor slab areas should be subjected to ample periods of heating to allow thawing of the soil system. Alternatively, the frozen soil can be completely removed and replaced with an engineered fill. The floor slab areas should be checked at random and representative locations for remnant areas of frost, and density tests should be performed to document fill compaction prior to slab placement.

Due to the potential problems associated with fill placement during cold weather, any filling operations should be monitored by a full-time, on-site soils technician. Full-time monitoring aids in detecting areas of frozen material, or potential problems with frozen material within the fill, so that appropriate measures can be taken. The choice of fill material is particularly important during cold weather, since clean granular fill materials can be placed and compacted more efficiently than silty or clayey soils. In addition, greater magnitudes of heaving can be expected with freezing of the more frost susceptible silts and clays.

If more specific frost information or cold weather data concerning other construction materials is required, please contact us.

BID FORM

PROJECT: New Town School Grandstands

BID OPENING DATE: September 10, 2026

BID OPENING TIME: 2:00pm CDT

BID OPENING PLACE: New Town High School Board Room
300 Eagle Drive
New Town, ND 58763

TO THE OWNER: New Town School
300 Eagle Drive
New Town, ND 58763

I have received the Drawings and Specifications for the _____ Contract for EAPC Project No. 20263400, New Town School Grandstands, to be erected at New Town, ND.

I have also received Addenda Nos. _____ and have included their provisions in my Bid. I have examined both the documents and the site and submit the following Bid:

I will do the _____ Construction Work for the lump sum price of _____ dollars (\$_____).

UNIT COSTS

I will include authorized unit cost work as specified to be added to or deducted from the above Base Bid, for changes in dollars and time listed:

Unit Cost No. 1, Unsatisfactory Soils (cubic yard)

ADD \$ _____

ADD _____ Calendar Days.

CONTINGENCY ALLOWANCE

I have included in the bid a contingency allowance of \$ _____ (complete with the appropriate amount from Secion 01 2100).

In submitting this Bid, I agree:

1. To hold my bid open for forty five (45) calendar days after its' opening.
2. To accept the provisions of the Instructions to Bidders regarding disposition of Bid Security.
3. To enter into and execute a Contract, if awarded on the basis of this bid, and to furnish Guarantee Bonds in accord with the Instructions to Bidders.
4. To accomplish the work in accord with the Contract Documents.
5. To substantially complete the Work in the Base Bid according to the following schedule:
To substantially complete the Work in the Base Bid by August 13, 2027.

I have attached the required Bid Security.

Official Address:	_____	Firm Name
_____	_____	signed by
_____	_____	title

SECTION 01 2100 - ALLOWANCES

PART 1 GENERAL

1.1 SECTION INCLUDES

- A. Contingency allowance.
- B. Payment and modification procedures relating to allowances.

1.2 RELATED REQUIREMENTS

- A. Section 01 2000 - Price and Payment Procedures: Additional payment and modification procedures.

1.3 CONTINGENCY ALLOWANCE

- A. Contractor's costs for products, delivery, installation, labor, insurance, payroll, taxes, bonding, equipment rental, overhead and profit will be included in Change Orders authorizing expenditure of funds from this Contingency Allowance.
- B. Funds will be drawn from the Contingency Allowance only by Change Order.
- C. At closeout of Contract, funds remaining in Contingency Allowance will be credited to Owner by Change Order.

1.4 ALLOWANCES SCHEDULE

- A. Contingency Allowance: Include the stipulated sum/price indicated below for use upon Owner's instructions.
 - 1. General Contract: \$165,000
 - 2. Mechanical Contract: \$50,000
 - 3. Electrical Contract: \$16,000
 - 4. Single Prime Contract: \$231,000

PART 2 PRODUCTS - NOT USED

PART 3 EXECUTION - NOT USED

END OF SECTION 01 2100

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GENERAL REQUIREMENTS:

- DESIGN AND CONSTRUCTION OF THIS PROJECT IS PER THE 2021 INTERNATIONAL BUILDING CODE (IBC) INCLUDING REFERENCED STANDARDS AND LOCAL/STATE AMENDMENTS OR BUILDING CODES.
- THIS PROJECT IS SUBJECT TO SPECIAL INSPECTIONS AS DESCRIBED IN CHAPTER 17 OF THE IBC AND SHALL BE PROVIDED BY AN INDEPENDENT AGENCY.
- GENERAL CONTRACTOR, CONTRACTOR, CONSTRUCTION MANAGER/AGENCY OR OTHER SIMILAR ENTITY OR REFERENCE ARE SYNONYMOUS AS IT RELATES TO USE OF AND RESPONSIBILITIES ASSIGNED BY THE CONTRACTOR DOCUMENTS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF STRUCTURAL SYSTEMS WITH OTHER TRADES AND CONTRACT DOCUMENTS, NOTIFY THE EOR OF ANY DISCREPANCIES.
- CONTRACTOR SHALL PROVIDE VERIFICATION OF QUANTITIES AND DIMENSIONS, MEANS AND METHODS OF CONSTRUCTION, SAFE AND SECURE PERFORMANCE OF THE WORK, AND TO NOT ALTER OR CUT ANY STRUCTURAL MEMBER WITHOUT APPROVAL FROM THE EOR.
- THE STRUCTURAL DESIGN REPRESENTS THE COMPLETED STATE ANY TEMPORARY BRACING OR SHORING REQUIRED TO MAINTAIN STABILITY DURING CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
- CONTRACTOR SHALL FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS, FRAMING MEMBERS, AND SITE CONDITIONS PRIOR TO STARTING THE WORK AND NOTIFY THE EOR OF ANY DISCREPANCIES.
- EXACT LOCATION AND SIZE OF ROOF/FLOOR/WALL OPENINGS, SLEEVES, OR MECHANICAL PENETRATIONS TO BE COORDINATED BY THE CONTRACTOR; SIMILAR FOR HOUSEKEEPING PADS, INSERTS AND EMBEDS, DEPRESSIONS, BOX-OUTS, SLOPES, DOORS AND WINDOWS, NON-BEARING WALLS, STAIRS, FINISHES, WATERPROOFING, RAILINGS, MECHANICAL EQUIPMENT LOCATIONS, LEDGES, INSULATION AND OTHER NON-STRUCTURAL ITEMS NECESSARY TO COMPLETE THE WORK.
- CONTRACTOR SHALL PROVIDE CONTRACTOR APPROVED AND STAMPED SHOP DRAWINGS AND PRODUCT DATA FOR REVIEW WITH GENERAL CONFORMANCE TO THE CONTRACTOR DOCUMENTS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFICATION AND COORDINATION WITH TESTING AND INSPECTION AGENCIES DURING CONSTRUCTION OR WHOEVER REMAINS RESPONSIBLE FOR PERFORMING WORK IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS ONLY.
- DEFERRED SUBMITTAL ITEMS REQUIRE SHOP DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED AS FOLLOWS:
 - METAL STAIRS AND CONNECTION TO STRUCTURE
 - LADDERS AND CONNECTION TO STRUCTURE
 - METAL RAILINGS/GUARDRAIL AND CONNECTION TO STRUCTURE
 - PRE-ENGINEERED WOOD TRUSSES
 - STRUCTURAL INSULATED PANELS (SIP)
 - MASS TIMBER FRAMING AND CONNECTIONS

SPECIAL INSPECTION AND TESTING:

- OWNER SHALL EMPLOY SERVICES OF AN INDEPENDENT TESTING AGENCY TO PERFORM SPECIFIED SPECIAL INSPECTION AND TESTING AS REQUIRED BY THE IBC OR AS SPECIFICALLY NOTED.
- SPECIAL INSPECTION AND TESTING SHALL BE COMPLETED IN ACCORDANCE WITH CHAPTER 17 OF THE IBC AND AS REQUIRED BY THE AUTHORITY HAVING JURISDICTION.
- CONTRACTOR SHALL PROVIDE SPECIAL INSPECTOR WITH SUFFICIENT NOTICE AND ACCESS TO THE SITE.
- CONTRACTOR SHALL BE RESPONSIBLE FOR THE COST OF RETESTING OR ADDITIONAL INSPECTION NEEDED AS A RESULT OF UNINSPECTED WORK, FAILED TESTS, OR REJECTED WORK.
- DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
 - OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS.
 - FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND PROJECT TEAM.
 - IMMEDIATELY NOTIFY THE CONTRACTOR AND ARCHITECT/EOR OF ANY DISCREPANCIES, DEVIATIONS, OR FAILED TEST FOR CORRECTIVE ACTION.

EARTHWORK AND SUBGRADE PREPARATION:

- COMPLETE EARTHWORK ACTIVITIES AND SITE PREPARATION IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT PREPARED BY **MITS & ASSOCIATES, INC.**
- NOTIFY EOR IMMEDIATELY IF ANY QUESTIONABLE SOIL CONDITIONS ARE ENCOUNTERED DURING EXCAVATION; FOUNDATION ELEVATIONS AND SUBGRADE PREPARATION ARE SUBJECT TO CHANGE DEPENDING ON CONDITIONS ENCOUNTERED.
- PROJECT GEOTECHNICAL ENGINEER SHALL TEST TO VERIFY DESIGN BEARING PRESSURE AND INSPECT SUBGRADE EXCAVATION AND PREPARATION PRIOR TO PROCEEDING WITH PLACEMENT OF FOUNDATIONS AND SLABS.
- FOUNDATION AND SLABS SHALL BEAR ON UNDISTURBED SOIL OR APPROVED FILL MATERIAL COMPACTED TO A MINIMUM OF 95% OF STANDARD PROCTOR UNLESS OTHERWISE NOTED OR SPECIFIED BY THE PROJECT GEOTECHNICAL ENGINEER.
- PROTECT EXCAVATIONS AND FOOTINGS FROM THE ACTION OF WATER OR FREEZING AND SHALL NOT BE EARTH FORMED WITHOUT APPROVAL BY THE EOR.
- CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AS REQUIRED TO COMPLY WITH OSHA STANDARDS.
- MATERIAL FOR BACKFILL SHALL BE CLEAN, FREE OF WOOD SCRAPS OR OTHER DELETERIOUS SUBSTANCES AND PLACED IN 12" COMPACTED LIFTS, UNLESS NOTED OR SPECIFIED BY THE PROJECT GEOTECHNICAL ENGINEER.
- BACKFILL AGAINST FOUNDATION WALL CAREFULLY TO AVOID DAMAGE TO THE WALL AND OTHER CONSTRUCTION; NO UNBALANCED BACKFILL SHALL BE PLACED UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY CONSTRUCTION BRACING OR PERMANENT CONSTRUCTION, OR DESIGNED TO RETAIN EARTH.
- EXISTING ON-SITE SUITABLE SOILS ARE ACCEPTABLE BACKFILL MATERIAL PROVIDED THEY CAN BE CONDITIONED AND PLACED TO THE SPECIFIED WATER CONTENT, MEET COMPACTION REQUIREMENTS, AND ARE APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER.
- GRANULAR FILL SHALL HAVE 100% PASSING THE 1", LESS THAN 50% PASSING THE #40, AND LESS THAN 12% PASSING THE #200 SIEVES UNLESS NOTED OR SPECIFIED BY THE PROJECT GEOTECHNICAL ENGINEER.
- RETAINING WALLS HAVE BEEN DESIGNED FOR THE LATERAL EARTH PRESSURE AND SURCHARGE NOTED, ASSUMING A WELL GRADED AND DRAINED CONDITION USING COMPACTED IMPORTED CLEAN SANDS OR SANDS WITH GRAVEL, AS DIRECTED BY GEOTECHNICAL ENGINEER, AT HEEL AND TOE SIDE OF FOUNDATION EXTENDING TOWARD FINISHED GRADE AT 1H-1V.
- PROVIDE A MINIMUM 6" THICK DRAINAGE COURSE BELOW ALL INTERIOR FLOOR SLABS ON GRADE UNLESS NOTED OTHERWISE. DRAINAGE COURSE SHALL HAVE 100% PASSING THE 1", LESS THAN 50% PASSING #40, AND LESS THAN 5% PASSING THE #200 SIEVES, UNLESS NOTED OR SPECIFIED BY THE PROJECT GEOTECHNICAL ENGINEER. PROVIDE A 10 MIL VAPOR BARRIER BETWEEN THE DRAINAGE COURSE AND UNDERSIDE OF CONCRETE SLAB.
- CRUSHED STONE/GRAVEL AROUND DRAINTILE SYSTEMS AND RETAINING WALL DRAINS SHALL HAVE LESS THAN 10% PASSING THE #4 SIEVE UNLESS NOTED OTHERWISE.

CAST-IN-PLACE CONCRETE:

- CONTRACTOR WORK SHALL CONFORM WITH REQUIREMENTS OF ACI 301 AND ACI 318 OR ACI 350, ALONG WITH ACI 305 AND ACI 306 WHEN WEATHER CONDITIONS DICTATE HOT OR COLD WEATHER PROVISIONS.
- CONCRETE SHALL BE A REDI-MIXED PRODUCT IN CONFORMANCE WITH APPROVED MIX DESIGN SUBMITTALS FOR EACH COMBINATION OF STRENGTH OR APPLICATION AS PREPARED BY THE SUPPLIER OR AND INDEPENDENT TESTING AGENCY.
- DO NOT USE ADMIXTURES OR CURING ACCELERATORS CONTAINING CALCIUM CHLORIDE.
- TOLERANCES AND FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED TO CONFORM WITH REQUIREMENTS OF ACI 114 AND ACI 347.
- CHAMFER EXPOSED EDGES OF CONCRETE 3/4" WHERE NOT SPECIFICALLY SHOWN OR NOTED ON STRUCTURAL OR ARCHITECTURAL DRAWINGS.
- SEE CONCRETE REINFORCING NOTES SECTION, STANDARD DETAILS AND TABLES FOR ADDITIONAL REINFORCING AND LAYOUT REQUIREMENTS.
- ALL FOOTINGS, FOUNDATIONS, PIERS, COLUMNS, AND FRAMING MEMBERS SHALL BE CENTERED UNLESS NOTED OTHERWISE.
- FOOTING STEPS SHALL BE GENERALLY LOCATED WHERE INDICATED ON THE FOUNDATION PLANS.
- PROVIDE DOWEL BARS BETWEEN MEMBERS WITH STANDARD HOOKS TO SUPPORT MEMBER AND LAP SPLICED WITH PRIMARY REINFORCING; DOWELS SHALL BE SAME SIZE AND SPACED TO MATCH PRIMARY REINFORCING UNLESS NOTED OTHERWISE.
- INSERTS, SLEEVES, OPENINGS, AND OTHER EMBEDDED ITEMS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE LOCATED AND VERIFIED BY THE CONTRACTOR; THOSE NOT SHOWN SHALL BE INSTALLED ONLY AFTER APPROVAL BY THE EOR.
- CONTRACTOR TO LAY OUT AND SUBMIT LOCATION OF CONSTRUCTION JOINTS AND CONCRETE CONSTRUCTION FOR REVIEW AND APPROVAL. SEE STANDARD DETAILS FOR SLAB ON GRADE CONSTRUCTION, CUT JOINTS WITHIN 24 HOURS OF CONCRETE PLACEMENT, SPACED NOT MORE THAN 36 TIMES THE SLAB THICKNESS, AND AN ASPECT RATIO NOT EXCEEDING 1.5.
- UNLESS SPECIFICALLY DETAILED OR SHOWN ON THE STRUCTURAL DRAWINGS, PENETRATIONS OR OPENINGS IN SLABS AND WALL SHALL:
 - NOT EXCEED 12 INCHES IN ANY DIMENSION.
 - NOT BE LOCATED WITHIN 6 INCHES FROM AN EDGE.
 - NOT BE LOCATED WITHIN A BEAM OR COLUMN.
 - NOT BE CLOSER THAN 10 TIMES THE SLAB THICKNESS FROM A CONCENTRATED LOAD EXCEEDING 2,000 LBS.
 - WHEN PLACED IN GROUPS, SHALL BE PLACED WITH NO LESS THAN ONE PENETRATION OF OPENING DIAMETER CLEAR BETWEEN THEM.

CONCRETE REINFORCEMENT:

- CONCRETE REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM WITH REQUIREMENTS OF ACI 315 AND CRSI MANUAL.
- SEE STANDARD DETAILS AND TABLES FOR ADDITIONAL INFORMATION RELATED TO STEEL REINFORCING COVER, LAPS, AND MISCELLANEOUS DETAILING.
- REINFORCING STEEL SHALL BE TIED, ACCURATELY PLACED, AND SUPPORTED BY STANDARD ACCESSORIES. ACCESSORY LEGS EXPOSED TO VIEW SHALL BE STAINLESS STEEL.
- REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE LOCATED AND HELD IN PLACE PRIOR TO CONCRETE PLACEMENT; PLACEMENT OR PUSHING ("WET STICKING") INTO CONCRETE IS NOT ALLOWED.
- REINFORCEMENT LAP SPLICES SHALL BE CLASS B PLUS 6" AT NON-CONTACT SLICES, UNLESS NOTED OTHERWISE, AND SPLICED AT LOCATIONS SHOWN OR NOTED WITHIN THE STRUCTURAL DOCUMENTS; EXCEPTION REINFORCEMENT NOTED AS "CONTINUOUS" MAY BE SPLICED AT LOCATIONS DETERMINED BY THE CONTRACTOR.
- DO NOT WELD REINFORCING UNLESS SPECIFICALLY SHOWN ON PLANS OR WITH WRITTEN PERMISSION FROM THE EOR.
- PROVIDE (2) #5'S, ONE EACH FACE, AROUND ALL OPENINGS GREATER THAN 12"x12", AND EXTENDING 2'-0" BEYOND OPENING IN BOTH DIRECTIONS; AT CORNERS, SIMILARLY PROVIDE (2) #5x4'-0" BARS DIAGONALLY.

CAST-IN-PLACE CONCRETE FIELD QUALITY CONTROL:

- TESTING AND INSPECTING:
 - CONTRACTOR SHALL ENGAGE A QUALIFIED TESTING AND INSPECTING AGENCY TO PERFORM FIELD TESTS AND INSPECTIONS AND PREPARE TEST REPORTS.
- CONCRETE TESTS:
 - TESTING OF COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C172 SHALL BE PERFORMED ACCORDING TO THE FOLLOWING REQUIREMENTS:
 - TESTING FREQUENCY: OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE EXCEEDING 5 CU. YD. (4 CU. M), BUT LESS THAN 25 CU. YD. (19 CU. M), PLUS ONE SET FOR EACH ADDITIONAL 50 CU.YD. (38 CU. M) OR FRACTION THERE OF.
 - WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN THREE COMPRESSIVE-STRENGTH TEST FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST THREE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
 - SUMP: ASTM C143/C 143M: ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
 - AIR CONTENT: ASTM C231, PRESSURE METHOD, FOR NORMAL-WEIGHT CONCRETE; ONE TEST FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE.
 - CONCRETE TEMPERATURE: ASTM C1064/C 1064M; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F (4.4 DEG C) AND BELOW AND WHEN 80 DEG F (27 DEG C) AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
 - COMPRESSION TEST SPECIMENS: ASTM C31/C31M.
 - CAST AND LABORATORY CURE TWO SETS (4 TOTAL) OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
 - COMPRESSIVE-STRENGTH TESTS: ASTM C39/C39M; TEST ONE SET OF TWO LABORATORY-CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO SPECIMENS AT 28 DAYS.
 - A COMPRESSIVE-STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM A SET OF TWO SPECIMENS OBTAINED FROM SAME COMPOSITE SAMPLE AND TESTED AT AGE INDICATED.
 - WHEN STRENGTH OF FIELD-CURED CYLINDERS IS LESS THAN 85 PERCENT OF COMPANION LABORATORY-CURED CYLINDERS, CONTRACTOR SHALL VALUATE OPERATIONS AND PROVIDE CORRECTIVE PROCEDURES FOR PROTECTING AND CURING IN-PLACE CONCRETE
 - STRENGTH OF EACH CONCRETE MIXTURE WILL BE SATISFACTORY IF EVERY AVERAGE OF ANY THREE CONSECUTIVE COMPRESSIVE-STRENGTH TEST EQUALS OR EXCEEDS SPECIFIED COMPRESSIVE STRENGTH AND NO COMPRESSIVE-STRENGTH TEST VALUE FALLS BELOW SPECIFIED COMPRESSIVE STRENGTH BY MORE THAN 500 PSI (3.4 MPa).
 - TEST RESULTS SHALL BE REPORTED IN WRITING TO ENGINEER, CONCRETE MANUFACTURER, AND CONTRACTOR WITHIN 48 HOURS OF TESTING. REPORTS OF COMPRESSIVE-STRENGTH TESTS SHALL CONTAIN PROJECT IDENTIFICATION NAME AND NUMBER, DATE OF CONCRETE PLACEMENT, NAME OF CONCRETE TESTING AND INSPECTING AGENCY, LOCATION OF CONCRETE BATCH IN WORK, DESIGN COMPRESSIVE STRENGTH, AND TYPE OF BREAK FOR BOTH 7 AND 28 DAY TESTS.
 - NON DESTRUCTIVE TESTING:
 - IMPACT HAMMER, SONOSCOPE, OR OTHER NON DESTRUCTIVE DEVICE MAY BE PERMITTED BY ENGINEER BUT WILL NOT BE USED AS SOLE BASIS FOR AN APPROVAL OR REJECTION OF CONCRETE.
 - ADDITIONAL TESTS:
 - TESTING AND INSPECTING AGENCY SHALL MAKE ADDITIONAL TEST OF CONCRETE WHEN TEST RESULTS INDICATE THAT SLUMP, AIR ENTRAINMENT, COMPRESSIVE STRENGTH, OR OTHER REQUIREMENTS HAVE NOT BEEN MET, AS DIRECTED BY ENGINEER. TESTING AND INSPECTING AGENCY MAY CONDUCT TEST TO DETERMINE ADEQUACY OF CONCRETE BY CORED CYLINDERS COMPLYING WITH ASTM C42/C42M OR BY OTHER METHODS AS DIRECTED BY ENGINEER.
 - ADDITIONAL TESTING AND INSPECTING, AT CONTRACTOR'S EXPENSE, WILL BE PERFORMED TO DETERMINE COMPLIANCE OF REPLACED OR ADDITIONAL WORK WITH SPECIFIED REQUIREMENTS.
 - CORRECT DEFICIENCIES IN THE WORK THAT TEST REPORTS AND INSPECTIONS INDICATE DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS.

POST INSTALLED ANCHORS:

- POST INSTALLED ANCHORS SHALL ONLY BE USED WHERE OR WHEN SPECIFIED IN THE CONTRACT DOCUMENTS.
- PRIOR APPROVAL FROM THE EOR IS REQUIRED BEFORE USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN ANCHORS, BOLTS, OR RODS.
- NO STEEL REINFORCEMENT SHALL BE CUT TO INSTALL ANCHORS, LOCATE PRIOR TO DRILLING HOLES.
- POST INSTALLED ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AND MAINTAIN NECESSARY EMBEDMENT, HOLE PREPARATION, EDGE DISTANCE, AND SPACING.
- INSPECTION AND TESTING SHALL BE PER THE GENERAL NOTES AND CHAPTER 17 OF THE IBC.
- SUBSTITUTION FOR A DIFFERENT TYPE OF ANCHOR OR VENDOR IS NOT PERMITTED WITHOUT A PRIOR SUBSTITUTION REQUEST SUBMITTAL WITH JUSTIFICATION AND COMPARATIVE CAPACITY ANALYSIS FOR APPROVAL BY THE EOR.

WOOD FRAMING:

- WOOD FRAMING AND CONSTRUCTION SHALL COMPLY WITH CONSTRUCTION DOCUMENTS, APA, AND/OR TPI STANDARD. GLULAM FABRICATOR SHALL BE AITC OR APA MEMBER AND FOLLOW APPEARANCE AND GRADE SPECIFICATIONS NOTED.
- PROVIDE WOOD FRAMING FASTENING PER THE IBC FASTENING SCHEDULE. COMMON NAILS, UNLESS NOTED OTHERWISE OR RECOMMENDED BY ENGINEERED WOOD SUPPLIER. MULTI PLY MEMBERS SHALL BE FASTENED WITH (2) ROWS OF 16d NAILS AT 12" O/C; FOR MEMBERS DEEPER THAN 12", PROVIDE (3) ROWS.
- FASTENERS TO BE STAGGERED TO PREVENT SPLITTING OF WOOD FRAMING. DRILLED BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. CUT WASHERS SHALL BE USED WHERE BOLT HEADS, NUTS, AND LAG SCREW HEADS BEAR ON WOOD. DO NOT NOTCH OR DRILL STRUCTURAL WOOD FRAMING EXCEPT AS ALLOWED BY THE IBC.
- WOOD NAILERS SHALL BE BOLTED TO STEEL FRAMING WITH (2) ROWS OF 1/2" DIAMETER A307 BOLTS SPACED 32" O/C, STAGGERED ROWS BY ONE HALF SPACING.
- TOP PLATES SHALL BE STAGGERED 48 INCHES MINIMUM AND FASTENED PLIES WITH (16) 16d COMMON NAILS EACH SIDE OF SPLICE
- SILL PLATES SHALL BE ANCHORED TO FOUNDATIONS WITH 5/8" DIAMETER BY 7" EMBEDMENT GALVANIZED ANCHOR RODS OR 5/8" DIAMETER x 10" EMBEDMENT SIMPSON TITEN HD SCREW ANCHORS AT 48" O/C, UNLESS NOTED OTHERWISE. PROVIDE A MINIMUM 3"x3"x 0.225" PLATE WASHER ON ALL SILL PLATE ANCHOR BOLTS. THERE SHALL BE AT LEAST (2) ANCHOR BOLTS PER PIECE SPACED NOT MORE THAN 6 FEET APART , WITH (1) BOLT LOCATED NOT MORE THAN 12" OR LESS THAN 4" FROM EACH END OF EACH PIECE. (REVIEW EMBED & WASHER SIZES)
- WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED AND PROTECTED, OR NATURALLY RESISTANT TO DECAY
- BRACE INTERIOR NON-BEARING WALLS TO BLOCKING OR FRAMING ABOVE PROVIDING CLIPS ALLOWING FOR VERTICAL MOVEMENT OF THE STRUCTURE.
- PROVIDE 2x FULL HEIGHT JAMB AND KING STUDS EACH SIDE OF WALL OPENINGS EQUAL TO THE NUMBER OF HEADER PLIES UNLESS OTHERWISE NOTED.
- PROVIDE 2x FULL HEIGHT JAMB STUDS DIRECTLY BELOW GIRDER TRUSS BEARING IN ADDITION TO SOLID BLOCKING AT JOIST/TRUSS CAVITIES. NUMBER OF STUDS SHALL EQUAL THE NUMBER OF GIRDER PLIES, UNLESS OTHERWISE NOTED
- PROVIDE SOLID BLOCKING EQUAL IN SIZE/NUMBER/WIDTH OF JAMBS AT JOIST/TRUSS CAVITIES BELOW BEAM/HEADER/GIRDER BEARING LOCATIONS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF PANELIZED WALL CONSTRUCTION. EOR TO REVIEW SUBMITTED FRAMING FOR GENERAL CONFORMANCE AND DESIGN INTENT, NOT FOR ACTUAL ENGINEERING OF PANEL OR COMPONENTS.
- LAMINATED WOOD SUPPLIER SHALL PROVIDE ALL HANGERS, HARDWARE, AND ACCESSORIES NECESSARY FOR CONNECTIONS ASSOCIATED WITH LAMINATED WOOD MEMBERS.
- WOOD TO STEEL CONNECTION, HANGERS, AND HARDWARE REQUIRED TO COMPLETE THE INSTALLATION OF LAMINATED WOOD SHALL BE FURNISHED BY THE STRUCTURAL STEEL SUPPLIER.
- SHEATHING AND/OR BRACING SHALL BE INSTALLED PRIOR TO CONSTRUCTION OF THE FLOOR OR ROOF ABOVE.
- SEE SHEAR WALL SCHEDULE AND DETAILS FOR ADDITIONAL ANCHOR ROD, HOLDOWN, AND OTHER ANCHORAGE/FASTENING REQUIREMENTS.
- SHEATHING SHEET SIZES SHALL BE 4'-0"x8'-0" (MIN) EXCEPT AT BOUNDARIES. INSTALL SHEETS WITH THE LONG DIMENSION OR STRENGTH AXIS OF THE PANEL PERPENDICULAR TO SUPPORTS AND CONTINUOUS OVER TWO OR MORE SPANS. STAGGER END JOINTS OF ADJACENT SHEETS.
- ROOF DIAPHRAGM 10 1/4" SIP PANEL CAPABLE OF 300 PLF ALLOWABLE SHEAR LOAD
- FLOOR DIAPHRAGM (BLOCKED) USE 23/32" RATED STURD-1-FLOOR T&G WITH A 24" SPAN RATING.
- FASTEN SHEATHING TO SUPPORTING MEMBERS AS FOLLOWS:
 - FLOOR SHEATHING (BLOCKED): FASTEN SHEATHING TO FLOOR FRAMING WITH A BEAD OF CONSTRUCTION ADHESIVE AND 10d RING SHANK NAILS @ 8" O/C AT BLOCKED PANEL EDGES AND AT DIAPHRAGM BOUNDARIES AND 12" O/C AT INTERMEDIATE SUPPORTS. DOUBLE LAYER DIAPHRAGMS SHALL BE INSTALLED WITH OFFSET PANEL PATTERN ONE LAYER AT A TIME WITH NAILING PATTERN SHOWN TO UNDERLYING SUPPORTS.
 - SHEARWALL SHEATHING: SEE SHEARWALL SCHEDULE



Architecture	Engineering
Interior Design	Industrial

TELEPHONE **701.461.7222**

112 No.Roberts Street, Suite 300, Fargo ND 58102

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CLIENT
NEW TOWN SCHOOL DISTRICT

PROJECT DESCRIPTION
NEW TOWN FOOTBALL FIELD IMPROVEMENTS

CITY	WATFORD CITY
STATE	ND

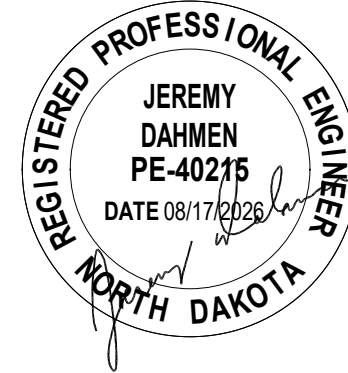
ISSUE DATES

1	ADDENDUM 01	08/26/2026
CD	CONSTRUCTION DOCUMENTS	08/17/2026
MARK	DESCRIPTION	DATE

PROJECT NO: 20263400
DRAWN BY: TLR/JCE
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DRAWING TITLE
STRUCTURAL GENERAL NOTES

S001

DESIGN CRITERIA AND LOADS (IN ADDITION TO THOSE INDICATED ON PLANS & DETAILS)		
OCCUPANCY	BUILDING RISK CATEGORY	III
DEAD LOADS	(SUPERIMPOSED)	
	ROOF	20 PSF
	FLOOR (MAIN LEVEL)	30 PSF (20 TC/10 BC)
	FLOOR (PRESS BOX)	25 PSF (15 TC/10 BC)
LIVE LOADS		
	PRESS BOX FLOOR	60 PSF
	MAIN FLOOR AND DECK	100 PSF
	STAIRWAYS	100 PSF
	ROOF (REDUCIBLE)	20 PSF
SNOW LOAD		
	GROUND SNOW LOAD	Pg 73 PSF
	SNOW EXPOSURE FACTOR	Ce 1.00
	THERMAL FACTOR (HEATED)	Ct 1.18
	THERMAL FACTOR (UNHEATED)	Ct 1.20
	ROOF SNOW LOAD	Pf 61 PSF
	SNOW DRIFT LOAD (ADDITIVE)	SEE PLAN
	WINTER WIND PARAMETER	W2 0.45
WIND DESIGN	(STRENGTH LEVEL UON)	
MAIN WIND FORCE RESISTING SYSTEM	BASIC WIND SPEED	V 120 MPH
	EXPOSURE CATEGORY	C
	BUILDING TYPE	ENCLOSED
	INTERNAL PRESSURE COEFFICIENT	GCpi +/-0.18
	UPLIFT ROOF (GROSS) OVERHANG	-49 PSF
	ROOF 0-6.5' (FROM ROOF EDGE)	-33 PSF
	ROOF > 6.5' (FROM ROOF EDGE)	-24 PSF
COMPONENTS AND CLADDING DESIGN PRESSURE		
	Ae ≤ 10 SQFT	Ae ≥ 500 SQFT
	ZONE 1	16.00/-40.00 PSF 16.00/-40.00 PSF
	ZONE 2	16.00/-46.00 PSF 16.00/-43.00 PSF
	ZONE 2'	16.00/-56.00 PSF 16.00/-52.00 PSF
	ZONE 3	16.00/-62.00 PSF 16.00/-43.00 PSF
	ZONE 3'	16.00/-86.00 PSF 16.00/-56.00 PSF
	ZONE 4	34.00/-37.00 PSF 26.00/-28.00 PSF
	ZONE 5	34.00/-45.00 PSF 26.00/-28.00 PSF
	ZONE 1 OH	16.00/-62.00 PSF 16.00/-54.00 PSF
	ZONE 2 OH	16.00/-69.00 PSF 16.00/-57.00 PSF
	ZONE 2' OH	16.00/-78.00 PSF 16.00/-66.00 PSF
	ZONE 3 OH	16.00/-84.00 PSF 16.00/-57.00 PSF
	ZONE 3' OH	16.00/-109.00 PSF 16.00/-69.00 PSF
	a = 3'-0"	
	Ae = AFFECTIVE AREA FOR C	

GEOTECHNICAL		
	ALLOWABLE SOIL BEARING PRESSURE fbrg	2000 PSF (ASSUMED)
	RETAINING WALL SURCHARGE	50 PSF
	LATERAL EARTH PRESSURE (PASSIVE, PROPERLY COMPACTED CLASS 5)	500 PSF/FT DEPTH
	PROPERLY COMPACTED IMPORTED CLEAN SANDS OR SANDS WITH GRAVEL	425 PSF/FT
	LATERAL EARTH PRESSURE (AT REST/ACTIVE, PROPERLY COMPACTED CLASS 5)	55/35 PSF/FT DEPTH
	PROPERLY COMPACTED IMPORTED CLEAN SANDS OR SANDS WITH GRAVEL	45/40 PSF/FT DEPTH
	COEFFICIENT OF FRICTION	0.30
MATERIAL STRENGTHS		
CONCRETE		
	28 DAY COMPRESSIVE STRENGTH	f'c
	FOOTINGS	4,000 PSI
	FOUNDATION WALLS/ PIERS/ WALLS/ COLUMNS/ SLABS/ BEAMS	4,000 PSI
	GROUT (NON-SHRINK / NON-METALLIC)	5,000 PSI (MIN)
	PROVIDE 6% (+/- 1.5%) ENTRAINED AIR IF EXTERIOR CONCRETE	
REINFORCING STEEL		
	REINFORCING BARS	ASTM A615 GR 60, DEFORMED
WOOD		
SAWN LUMBER	BEAMS AND STRINGERS	DFL No. 2
	STUD WALLS (BEARING)	SPF No. 2
	TOP AND BOTTOM PLATE (BEARING WALLS)	SOUTHERN PINE No. 2, TREATED
	MISCELLANEOUS FRAMING AND BLOCKING	SPF STUD GRADE
	DECK	SP No. 1 UC4A WATERBORNE PRESSURE TREATED
	JOISTS	SP No. 1 UC4A WATERBORNE PRESSURE TREATED
ENGINEERED LUMBER	POSTS AND TIMBERS & BRACING	SP No. 1 UC4A WATERBORNE PRESSURE TREATED
	MICROLAM LVL	Fb=2600 PSI & E=2000 KSI
	PARALLAM PSL	Fb=2900 PSI & E=2000 KSI
	TIMBER STRAND LSL	Fb=1700 PSI & E=1300 KSI
	GLULAM BEAM	SP 24F-V4 WATERBORNE PRESSURE TREATED UC4A
	GLULAM COLUMN	SP N2M12 WATERBORNE PRESSURE TREATED UC4A 4 LAMINATIONS
SHEATHING	WALL (SEE SHEAR WALL SCHEDULE FOR ADDITIONAL REQUIREMENTS)	7/16" APA RATED, EXPOSURE 1, 24/16 SPAN RATING
	FLOOR	3/4" APA RATED, STURD J FLOOR TONGUE & GROOVE, EXPOSURE 1, 24" SPAN RATING
SILL PLATE	ANCHORS	5/8"Ø SIMPSON TITEN HD SCREW ANCHORS (GALV)
POST INSTALLED ANCHORS**		
ADHESIVE ANCHORS	SIMPSON AT-3G	ANCHORAGE TO CONCRETE
CONCRETE ANCHORS	SIMPSON TITEN HD	ANCHORAGE TO CONCRETE & MASONRY
	**OR APPROVED EQUALS	

CONCRETE REINFORCING STEEL LAP SPLICE & DEVELOPMENT LENGTH SCHEDULE				
	MINIMUM LAP SPLICE LENGTHS (Ls)		MINIMUM DEVELOPMENT LENGTH (Ld)	
BAR SIZE	TOP BARS¹	OTHER BARS	TOP BARS¹	OTHER BARS
#3	2'-0"	1'-7"	1'-7"	1'-3"
#4	2'-8"	2'-1"	2'-1"	1'-7"
#5	3'-4"	2'-7"	2'-7"	2'-0"
#6	4'-0"	3'-1"	3'-1"	2'-5"
#7	5'-10"	4'-6"	4'-6"	3'-6"
#8	6'-8"	5'-2"	5'-2"	3'-11"
#9	7'-7"	5'-10"	5'-10"	4'-6"
1. HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM.				
CAST IN PLACE CONCRETE (NON-PRESTRESSED) COVER				
ACI 318 - STRUCTURAL CONCRETE				
UNLESS NOTED OTHERWISE ON DRAWINGS				COVER (INCHES)
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH				3
EXPOSED TO EARTH OR WEATHER				
	#6 THROUGH #18 BARS			2
	#5 AND SMALLER			1 1/2
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND				
SLABS, WALLS, JOISTS	#14 & #18 BARS			1 1/2
	#11 BARS AND SMALLER			3/4
BEAMS, COLUMNS	PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS			1 1/2
	SLAB ON GRADE / SLAB ON METAL DECK			CENTERED

REQUIRED SPECIAL INSPECTION OF CONCRETE CONSTRUCTION ¹		
VERIFICATION & INSPECTION	CONTINUOUS	PERIODIC
1. INSPECTION OF REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	--	X
2. REINFORCING BAR WELDING:	--	--
A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	--	X
B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND	--	X
C. INSPECT ALL OTHER WELDS	X	--
3. INSPECTION OF ANCHORS CAST INTO CONCRETE	--	X
4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS	--	X
A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARD INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X	--
B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE	--	X
5. VERIFY USE OF REQUIRED DESIGN MIX	--	X
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	--
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	--
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	--	X
9. INSPECTION OF PRESTRESSED CONCRETE FOR:	--	--
A. APPLICATION OF PRESTRESSING FORCED, AND	N/A	--
B. GROUTING OF BONDED PRESTRESSING	N/A	--
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	--	N/A
11. VERIFICATION IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	--	N/A
12. INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS PRIOR TO CONCRETE POUR	--	X
NOTE 1: ALL SPECIAL INSPECTIONS IN ACCORDANCE WITH CURRENT IBC AND ACI STANDARDS		

REQUIRED SPECIAL INSPECTION OF SOIL ¹		
VERIFICATION & INSPECTION	CONTINUOUS	PERIODIC
1. VERIFY MATERIAL BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE DESIGN BEARING CAPACITY	--	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	--	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	--	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	--
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	--	X
NOTE 1: ALL SPECIAL INSPECTIONS IN ACCORDANCE WITH CURRENT IBC		

REQUIRED SPECIAL INSPECTION AND TESTS FOR WIND RESISTANCE ON WIND-RESISTING COMPONENTS ¹		
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. FASTENING OF ROOF COVERING, ROOF DECK, ROOF FRAMING CONNECTIONS, EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGM AND FRAMING	--	X
NOTE 1: ALL SPECIAL INSPECTIONS IN ACCORDANCE WITH CURRENT IBC AND ACI STANDARDS		

REQUIRED SPECIAL INSPECTION AND TESTS OF WOOD CONSTRUCTION ¹		
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. SPECIAL INSPECTION OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5 OF THE IBC 2018	--	X
2. OWNER WILL ENGAGE A QUALIFIED SPECIAL INSPECTOR TO VERIFY TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE	--	X
NOTE 1: ALL SPECIAL INSPECTIONS IN ACCORDANCE WITH CURRENT IBC AND ACI STANDARDS		

REQUIRED SPECIAL INSPECTION AND TESTS FOR WIND RESISTANCE ON STRUCTURAL WOOD ¹		
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. DURING FIELD GLUING OPERATIONS OF THE MAIN WIND FORCE-RESISTING SYSTEM	X	--
2. FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLDDOWNS	--	X
NOTE 1: ALL SPECIAL INSPECTIONS IN ACCORDANCE WITH CURRENT IBC AND ACI STANDARDS		



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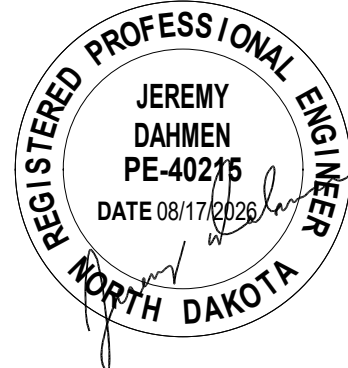
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STRUCTURAL LOADS, MATERIAL NOTE
TABLES AND SPECIAL INSPECTION TABLES

S002

SHEARWALL SCHEDULE

MARK	SHEATHING	EDGE	INTERMEDIATE	MEMBER REQUIREMENTS AT END OF SHEARWALLS	HOLD-DOWN & ANCHOR ROD AT EACH END	SILL PLATE FASTENING
SW-1: SIDE 1	7/16" APA RATED (SPAN RATING 24/16) EXPOSURE 1 PANEL EDGES BLOCKED ATTACHED TO STUD 5/8" GYPSUM BOARD OVER BLOCKED	8d @ 4" O/C / 6d COOLER NAILS @ 7" O/C	8d @ 12" O/C / 6d COOLER NAILS @ 7" O/C	6x6 DF-L No. 2	SIMPSON HDUES W/ 5/8" THREADED ROD	(2) ROWS SIMPSON SDS25300 @ 6" O/C
SW-1: SIDE 2	5/8" GYPSUM BOARD BLOCKED	6d COOLER NAILS @ 7" O/C	6d COOLER NAILS @ 7" O/C			

CONTINUOUS FOOTING SCHEDULE

MARK	FOOTING SIZE		REINFORCING		REMARKS
	WIDTH	THICKNESS	LONGITUDINAL	TRANSVERSE	
WF-1	1'-4"	12"	(2) #5 CONT	-	
WF-2	2'-6"	12"	(3) #5 CONT	-	
WF-3	15'-4"	24"	#6 @ 18" O/C TOP AND BTM	#7 @ 8" O/C TOP AND BTM	16" WIDE x 2'-6" DEEP SHEAR KEY UNDERNEATH FOOTING, EXTEND VERTICAL REBAR INTO KEY, SEE DETAIL 1/S602
WF-4	15'-0"	18"	#6 @ 18" O/C TOP AND BTM	#7 @ 6" O/C TOP AND BTM	
WF-5	11'-6"	18"	#6 @ 18" O/C TOP AND BTM	#6 @ 12" O/C TOP AND BTM	
WF-6	8'-0"	18"	#6 @ 18" O/C TOP AND BTM	#6 @ 8" O/C TOP AND BTM	
WF-7	9'-6"	18"	#6 @ 18" O/C TOP AND BTM	#6 @ 8" O/C TOP AND BTM	
WF-8	14'-0"	18"	#6 @ 18" O/C TOP AND BTM	#6 @ 8" O/C TOP AND BTM	

WOOD COLUMN SCHEDULE

MARK	SIZE	TYPE	BASE	CAP	REMARKS
C-1	8x8	SP No. 1 LUMBER	SIMPSON ABU88Z	SIMPSON CCQ-HDG	
C-2	8 1/2"x12 3/8"	GLULAM	SIMPSON CB		
C-3	(4) 2x6	SP No. 1 LUMBER	SIMPSON HDU9-SD3.5/ABU66RZ	SIMPSON CCQ-HDG STRAPS ROTATED 90	
C-4	6x14	DF-L No. 2 LUMBER	SIMPSON MSTC66B3Z L=48"	COLUMN TO BEAM (4) SIMPSON SDWC15600 DRIVEN 2" FROM END OF MEMBER AT 45° FROM COLUMN INTO BEAM	
C-5	(3) 2x6	DF-L No. 2 LUMBER	SIMPSON MSTC48B3	SIMPSON MSTC48B3	
C-6	8x8	DF-L No. 2 LUMBER	SIMPSON HDUE9-SDS3.5 W/ 7/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	
C-7	6x6	DF-L No. 2 LUMBER	SIMPSON HDUE5-SDS3 W/ 5/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	

THICKENED SLAB SCHEDULE

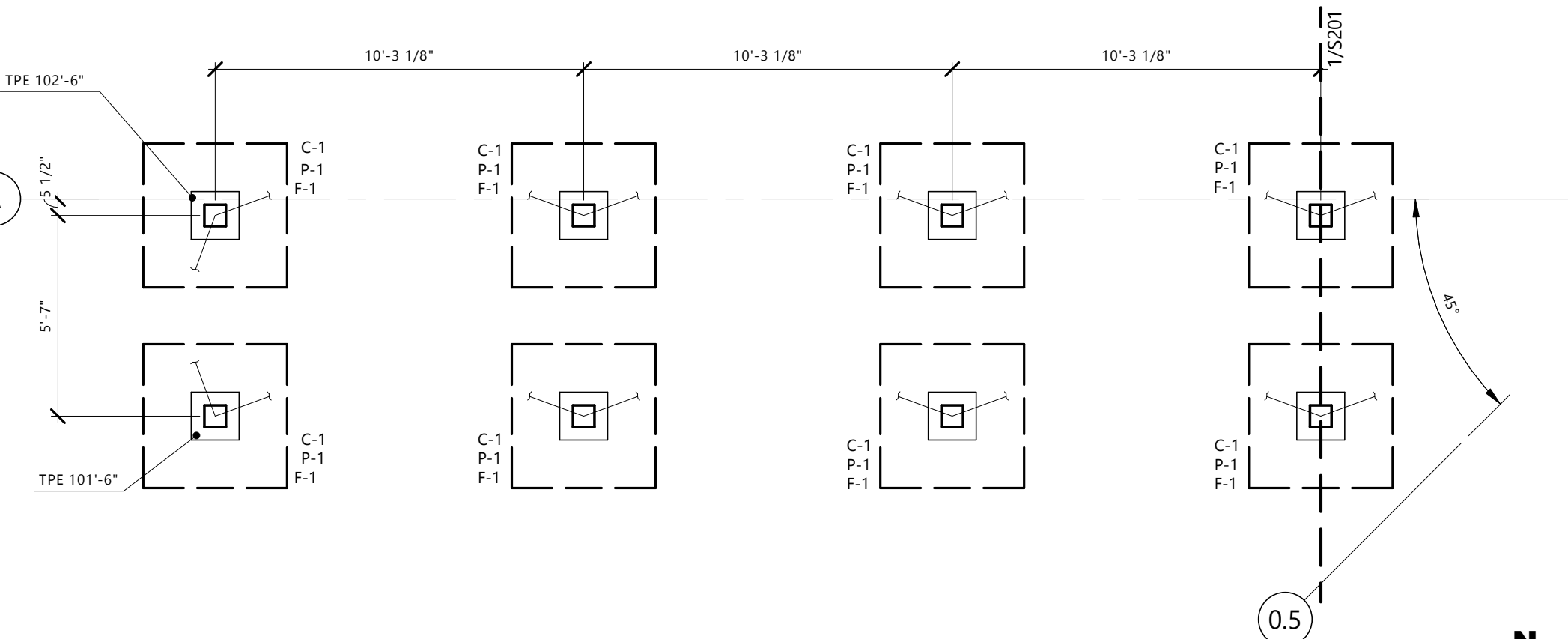
MARK	THICKENED SLAB SIZE		REINFORCING	REMARKS
	WIDTH	THICKNESS		
TS-1	5'-0"	24"	(6) #6 EACH WAY TOP & BOT	

FOUNDATION WALL SCHEDULE

MARK	WIDTH	MATERIAL	REINFORCING		REMARKS
			VERTICAL	HORIZONTAL	
W-1	8"	CONC	#5 @ 12" O/C - CENTERED	#5 @ 12" O/C - CENTERED	
W-2	10"	CONC	#5 @ 12" O/C - CENTERED	#5 @ 12" O/C - CENTERED	
W-3	1'-0"	CONC	#6 @ 8" O/C EA FACE	#6 @ 18" O/C EA FACE	
W-4	1'-0"	CONC	#7 @ 8" O/C EA FACE	#6 @ 18" O/C EA FACE	
W-5	1'-0"	CONC	#6 @ 12" O/C EA FACE	#6 @ 18" O/C EA FACE	
W-6	1'-4"	CONC	#7 @ 6" O/C EA FACE	#6 @ 18" O/C EA FACE	

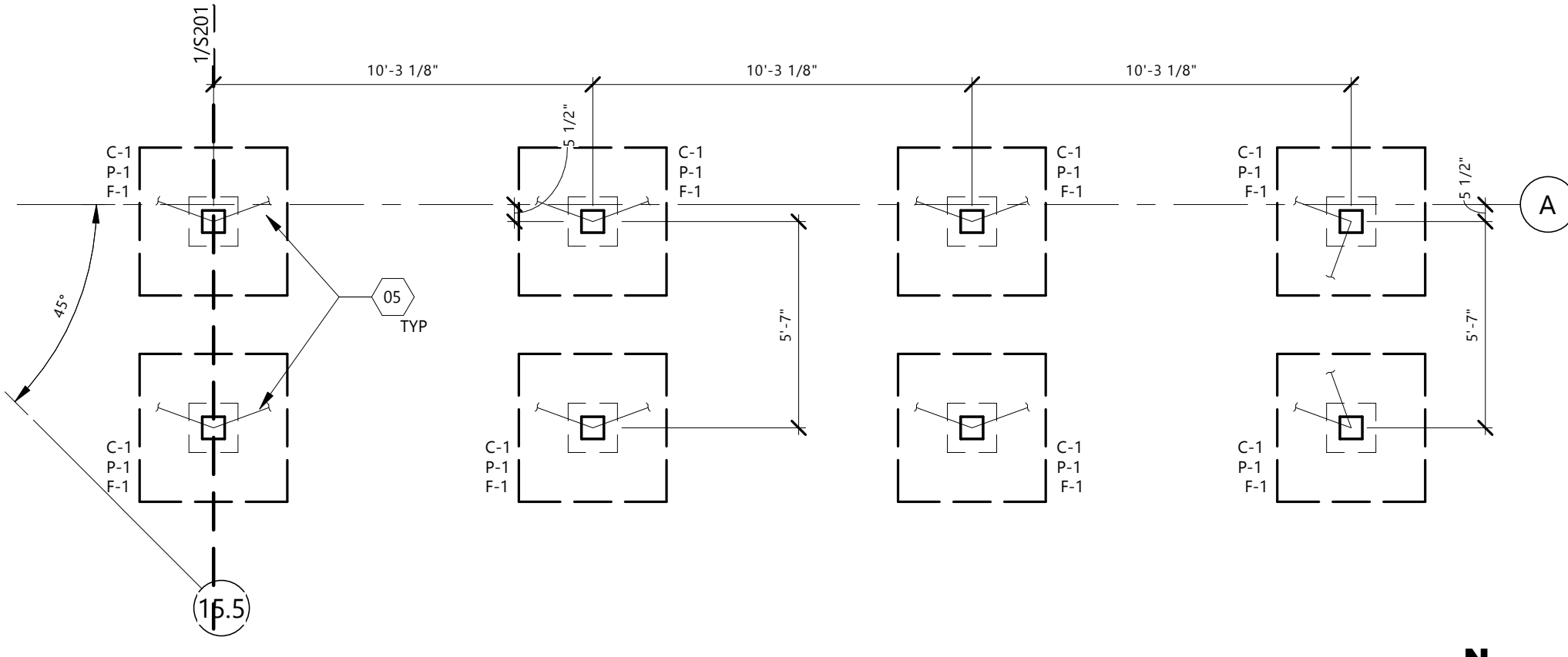
KEYNOTE LEGEND:

- △ < < INDICATES KEYNOTE ON PLAN
- 4" THICK CONCRETE SLAB-ON-GRADE OVER 10 MIL VAPOR BARRIER OVER 6" COMPACTED SAND DRAINAGE COURSE. REINFORCE SLAB W/ #3 @ 18" O/C EACH WAY, CENTERED IN SLAB.
 - PROVIDE 4" THICKENED WALL AT STOOP WITH DROPPED TOP OF CONCRETE FNDN WALL (-8" UON) AT EXTERIOR FACE, AND WIDEN 4" WITHIN STOOP SLAB TO PROVIDE BEARING LEDGE.
 - SEE DETAIL 10/S601 FOR COLUMN ISOLATION JOINT.
 - NOT USED
 - 4x6 X BRACING W/ (A) 3/4" GALVANIZED BOLTS THROUGH TOP AND BOTTOM OF COLUMN AND (1) THROUGH BOLT AT MID BRACE INTERSECTION.
 - FULL HEIGHT CONCRETE WALL. SEE ARCH AND MECH FOR OPENING BLOCKOUTS. REFER TO S601 NOTE 7 UNDER CONCRETE REINFORCEMENT FOR REINFORCEMENT AROUND OPENINGS. INSTALL TREATED 4x8 (3.5" DEEP x 7.25" WIDE) SILL PLATE AT TOP WITH 5/8" SIMPSON TITEN HD FLAT WASHER W/ 4" NOM EMBED @ 12" O/C. FASTEN EACH LAYER OF SILL FLOOR TO SILL WITH (3) STAGGERED ROWS 16d COMMON NAILS @ 6" O/C.
 - DOWEL #5 BARS 6" INTO WALL WITH SIMPSON AT 3G AND LAP WITH HORIZONTAL BARS IN WALL.
 - 2x8 TREATED PLATE NAILER TO VERTICAL EDGE OF WALL WITH 5/8" SIMPSON TITEN HD FLAT WASHER W/ 4" NOM EMBED @ 12" O/C.
 - RECESSED SLAB SEE ARCH FOR LOCATION AND EXTENTS.
 - HOLD DOWN LOCATION SEE COLUMN OR SHEARWALL SCHEDULE FOR TYPE.
 - HOLD DOWN LOCATION SEE SIP PANEL DESIGN FOR TYPE.
 - FILL IN BETWEEN FOOTINGS WITH CONCRETE.
 - COORDINATE SEATING AND RETAINING FOUNDATIONS. BOTTOM OF FOUNDATIONS SHALL BE AT SAME ELEVATION. BLOCK OUT RETAINING FOUNDATION FOR SEATING FOUNDATION.



3 LOWER LEVEL FOUNDATION/SLAB PLAN - LEFT SIDE DECK

S201 1/4" = 1'-0"



2 LOWER LEVEL FOUNDATION/SLAB PLAN - RIGHT SIDE DECK

S201 1/4" = 1'-0"

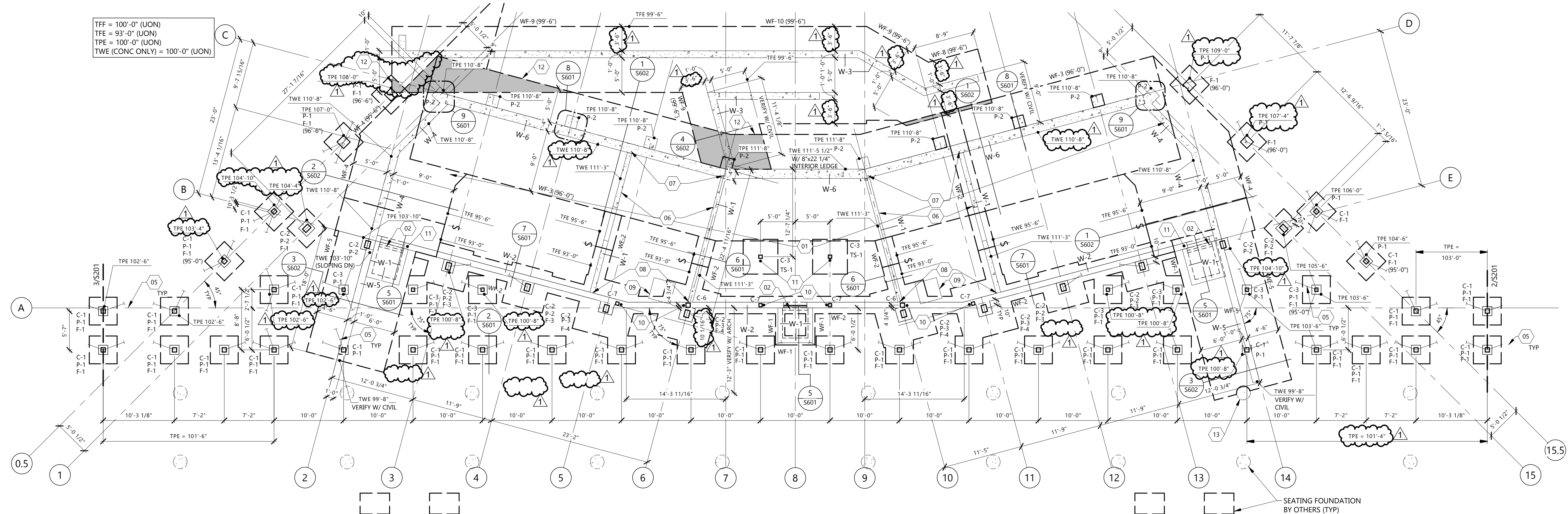
SPREAD FOOTING SCHEDULE

MARK	FOOTING		REINFORCING	REMARKS
	SIZE	THICKNESS		
F-1	4'-0" x 4'-0"	12"	(5) #5 EACH WAY TOP AND BTM	
F-3	6'-0" x 6'-0"	12"	(7) #5 EACH WAY TOP AND BTM	
F-4	8'-0" x 8'-0"	12"	(9) #5 EACH WAY TOP AND BTM	

CONCRETE PIER SCHEDULE

MARK	PIER SIZE	REINFORCING		REMARKS
		VERTICAL	TIE SIZE/SPACING	
P-1	16" x 16"	(4) #6	(2) #3 WITHIN TOP 8", REMAINDER @ 12" O/C	
P-2	20" x 20"	(8) #5	(2) #3 WITHIN TOP 8", REMAINDER @ 12" O/C	
P-3	24" x 24"	(8) #6	(2) #3 WITHIN TOP 8", REMAINDER @ 12" O/C	

- NOTES:
- CIP CONCRETE PIER CAST MONOLITHICALLY WITH FOUNDATION WALL WHERE APPLICABLE.
 - TYPICAL HORIZONTAL WALL REINFORCING TO RUN CONTINUOUS THROUGH PIER WHERE APPLICABLE.
 - PLACE PIER SUCH THAT COLUMN ANCHOR RODS & REINF ARE CONTAINED WITHIN PIER TIES.
 - BLOCKOUT FOUNDATION WALL AS REQUIRED FOR INSTALLATION OF STEEL BASE PLATE/COLUMN CONTRACTOR TO COORDINATE LAYOUTS. COORDINATE HOLD DOWNS AND EMBEDDED COLUMN BASES WITH FRAMING CONTRACTOR.
 - SEE 11/S601 FOR REINFORCEMENT AND CROSS TIE LAYOUT



1 LOWER LEVEL FOUNDATION/SLAB PLAN

S201 1/8" = 1'-0"

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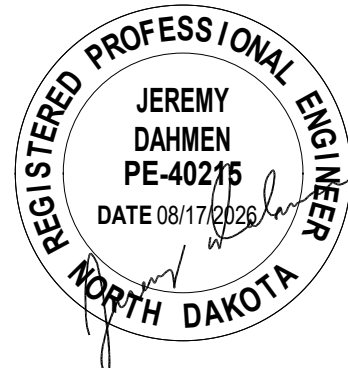
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LOWER LEVEL
FOUNDATION/SLAB
PLAN

S201

SHEARWALL SCHEDULE

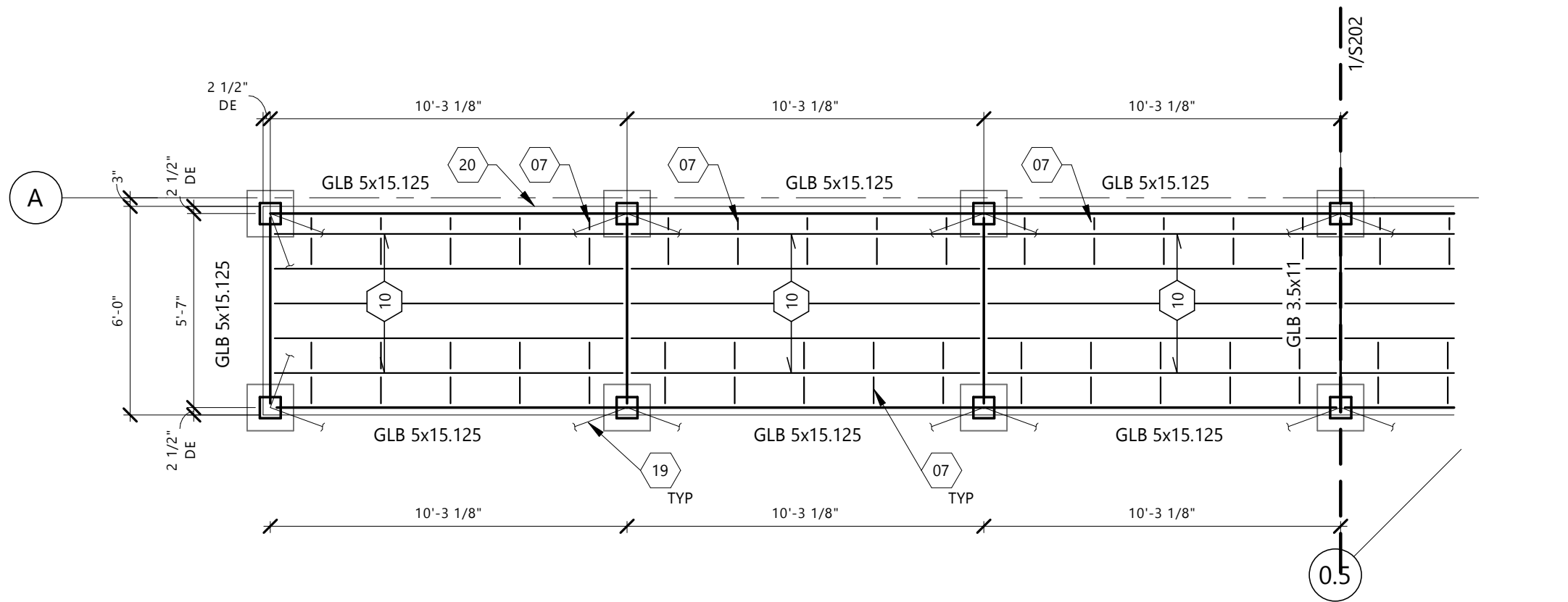
MARK	SHEATHING	NAILING (SIZING & SPACING)		MEMBER REQUIREMENTS AT END OF SHEARWALLS	HOLD-DOWN & ANCHOR ROD AT EACH END	SILL PLATE FASTENING
		EDGE	INTERMEDIATE			
SW-1: SIDE 1	7/16" APA RATED (SPAN RATING 24/16) EXPOSURE 1 PANEL EDGES BLOCKED ATTACHED TO STUD 5/8" GYPSUM BOARD OVER BLOCKED	8d @ 4" O/C / 6d COOLER NAILS @ 7" O/C	8d @ 12" O/C / 6d COOLER NAILS @ 7" O/C	6x6 DF-L No. 2	SIMPSON HDUE5 W/ 5/8" THREADED ROD	(2) ROWS SIMPSON SDS25300 @ 6" O/C
SW-1: SIDE 2	5/8" GYPSUM BOARD BLOCKED	6d COOLER NAILS @ 7" O/C	6d COOLER NAILS @ 7" O/C			

WOOD COLUMN SCHEDULE

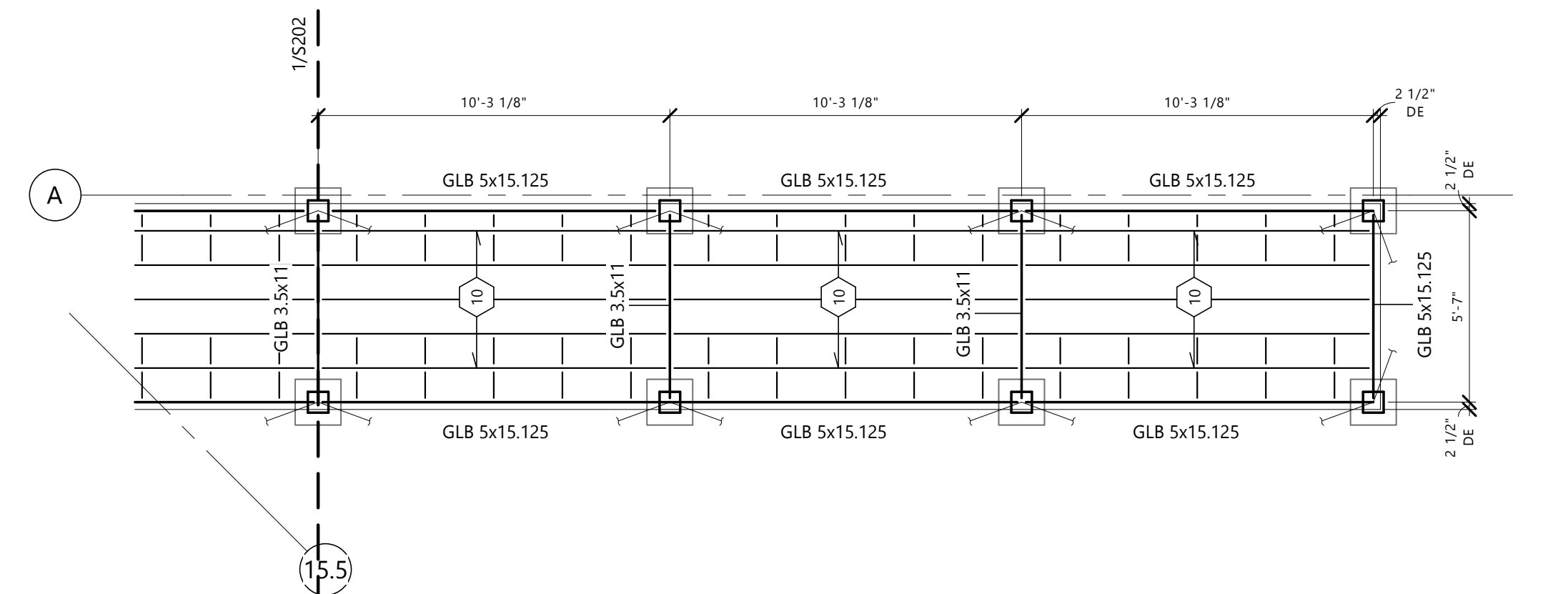
MARK	SIZE	TYPE	BASE	CAP	REMARKS
C-1	8x8	SP No. 1 LUMBER	SIMPSON ABU88Z	SIMPSON CCQ-HDG	
C-2	8 1/2"x12 3/8"	GLULAM	SIMPSON CB		
C-3	(4) 2x6	SP No. 1 LUMBER	SIMPSON HDU9-SD3.5/ABU66RZ	SIMPSON CCQ-HDG STRAPS ROTATED 90	
C-4	6x14	DF-L No. 2 LUMBER	SIMPSON MSTC66B3Z L=48"	COLUMN TO BEAM (4) SIMPSON SDWC15600 DRIVEN 2" FROM END OF MEMBER AT 45° FROM COLUMN INTO BEAM	
C-5	(3) 2x6	DF-L NO. 2 LUMBER	SIMPSON MSTC48B3	SIMPSON MSTC48B3	
C-6	8x8	DF-L NO. 2 LUMBER	SIMPSON HDUE9-SDS3.5 W/ 7/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	
C-7	6x6	DF-L NO. 2 LUMBER	SIMPSON HDUE5-SDS3 W/ 5/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	

KEYNOTE LEGEND:

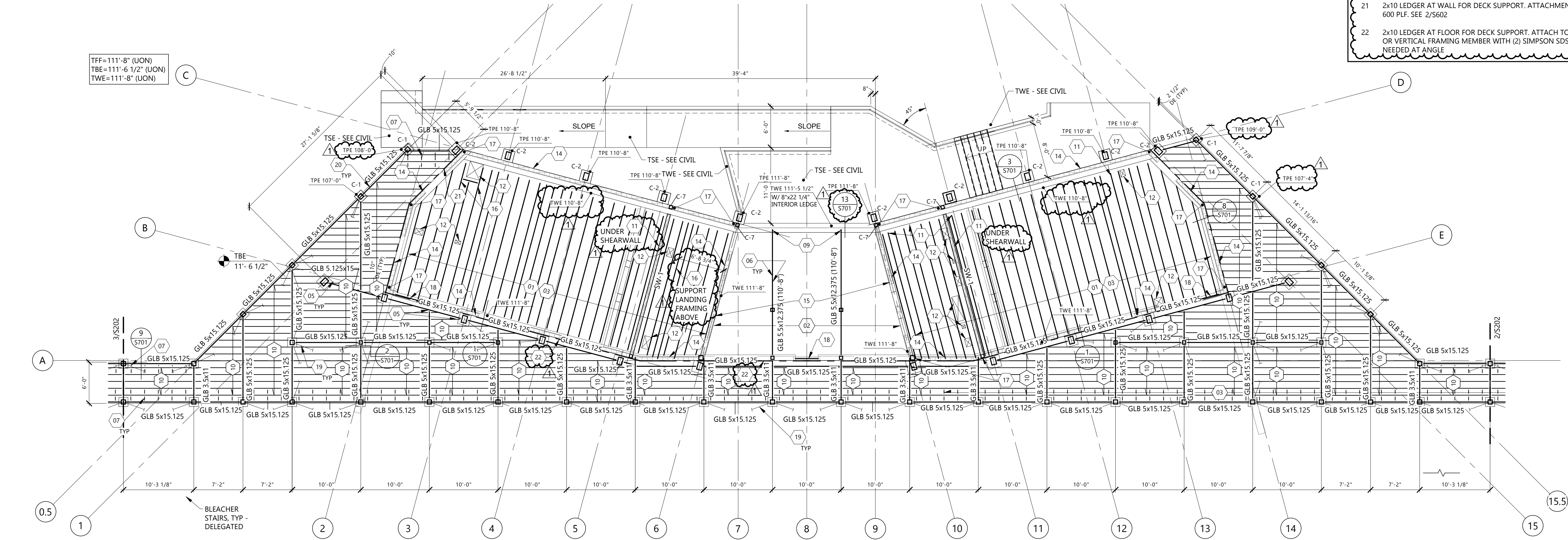
< < INDICATES KEYNOTE ON PLAN	01	20" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 19.2" O/C.
	02	10 1/4" STRUCTURAL INSULATED PANEL ROOF TYPE L SPLINE W/ 2x SLEEPER FRAMING AND PEDESTAL DECK SYSTEM
	03	(2) LAYERS 3/4" TONGUE AND GROOVE APA RATED (SPAN RATING 48/24) STURD-I-FLOOR SHEATHING (BLOCKED). SEE S001 FOR SHEATHING FASTENING REQUIREMENTS
	04	SIMPSON HDUE AT COLUMN ABOVE AND BELOW FLOOR. SEE COLUMN SCHEDULE FOR SIZE
	05	SIMPSON CBH (GALVANIZED) AT BEAM TO GLULAM COLUMN & BEAM TO BEAM CONNECTIONS.
	06	(1) 2x10 SP No. 1 LEDGER ATTACH TO GLULAM WITH (2) HORIZONTAL ROWS SIMPSON SDS25300 SPACED 6" O/C. EACH SIDE STAGGERED
	07	INSTALL 2x10 BLOCKING SPACED 2' O/C IN FIRST 2 JOIST SPACES WHERE GUARD RAIL INSTALLED. ATTACH BLOCKING TO JOIST WITH SIMPSON LUS28 AND DECKING TO BLOCKING WITH 6" O/C FASTENERS
	08	NOT USED
	09	BEAM BEARING WITH SIMPSON HGLB IN WALL
	10	2x10 SP No1 JOISTS @ 12" O/C. PRESSURE TREATED LUMBER (USE CATEGORY UC4A) W/ SIMPSON LUS28 TYP & SIMPSON HU28 AS NEEDED AT SKEW LOCATIONS.
	11	TRUSS SUPPLIER ADD FLOOR TRUSS AND SHIFT LOCATION AS NEEDED FOR PLUMBING OR MECHANICAL
	12	MECH OPENING - VERIFY LOCATION AND EXACT SIZE WITH MECH. FASTEN SHEATHING W/ EDGE NAILING PATTERN AND BLOCKING SUPPORTED BY ADJACENT TRUSSES AROUND OPENING. TRUSS SUPPLIER ADD TRUSS AS NEEDED TO MAINTAIN SPACING
	13	NOT USED
	14	6 1/2" STRUCTURAL INSULATED PANEL WALL
	15	(2) 2x LEDGER FASTENED TO CONC WALL W/ 3/4"Ø SIMPSON TITEN HD NOM EMBED 5" SPACED @ 12" O/C MIN 2" ABOVE MECHANICAL OPENINGS. TOP OF LEDGER EL = 109'-11 3/4".
	16	PRE-ENGINEERED FLOOR TRUSS GIRDER
	17	HOLD-DOWN LOCATION. SEE COLUMN OR SHEARWALL SCHEDULE FOR TYPE
	18	SIP HEADER DESIGN BY SIP SUPPLIER
	19	4x6 X-BRACING W/ (2) 3/4" GALVANIZED BOLTS THROUGH TOP AND BOTTOM OF COLUMN WITH BLOCK AND 1" THROUGH BOLT AT MID BRACE INTERSECTION.
	20	GUARD-RAIL - SEE ARCH - ATTACH TO OUTSIDE OF DECK BEAM. DESIGN AND CONNECTION BY SUPPLIER
	21	2x10 LEDGER AT WALL FOR DECK SUPPORT. ATTACHMENT BY SIP SUPPLIER FOR MIN 600 PLF. SEE 2/S602
	22	2x10 LEDGER AT FLOOR FOR DECK SUPPORT. ATTACH TO END OF EACH FLOOR TRUSS OR VERTICAL FRAMING MEMBER WITH (2) SIMPSON SDS25300. PROVIDE BLOCKING AS NEEDED AT ANGLE



3 MAIN LEVEL FRAMING PLAN - LEFT SIDE DECK
 1/4" = 1'-0"



2 MAIN LEVEL FRAMING PLAN - RIGHT SIDE DECK
 1/4" = 1'-0"



1 MAIN LEVEL FRAMING PLAN - MAIN BUILDING
 1/8" = 1'-0"

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PROJECT DESCRIPTION
 NEW TOWN FOOTBALL FIELD IMPROVEMENTS

CITY WATFORD CITY
 STATE ND

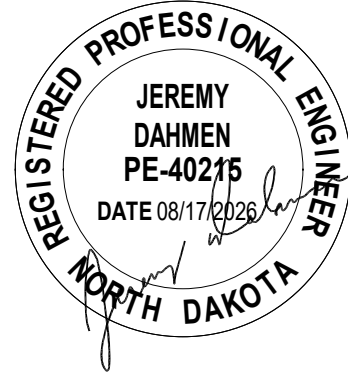
ISSUE DATES

1	ADDENDUM 01	08/26/2026
CD	CONSTRUCTION DOCUMENTS	08/17/2026
MARK	DESCRIPTION	DATE

PROJECT NO: 20263400
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DRAWING TITLE
 MAIN LEVEL FRAMING PLAN

S202

WOOD COLUMN SCHEDULE

MARK	SIZE	TYPE	BASE	CAP	REMARKS
C-1	8x8	SP No. 1 LUMBER	SIMPSON ABU88Z	SIMPSON CCQ-HDG	
C-2	8 1/2"x12 3/8"	GLULAM	SIMPSON CB		
C-3	(4) 2x6	SP No. 1 LUMBER	SIMPSON HDU9-SD3.5/ABU66RZ	SIMPSON CCQ-HDG STRAPS ROTATED 90	
C-4	6x14	DF-L No. 2 LUMBER	SIMPSON MSTC66B3Z L=48"	COLUMN TO BEAM (4) SIMPSON SDWC15600 DRIVEN 2" FROM END OF MEMBER AT 45° FROM COLUMN INTO BEAM SIMPSON MSTC48B3	
C-5	(3) 2x6	DF-L NO. 2 LUMBER	SIMPSON MSTC48B3		
C-6	8x8	DF-L NO. 2 LUMBER	SIMPSON HDUE9-SDS3.5 W/ 7/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	
C-7	6x6	DF-L NO. 2 LUMBER	SIMPSON HDUE5-SDS3 W/ 5/8" THREADED ROD SET 6" INTO FOUNDATION W/ SIMPSON AT-3G	SIMPSON HDUE9-SDS3.5/CCQ-HDG	

WOOD BEAM SCHEDULE

MARK	SIZE	MATERIAL	REMARKS
B-1	5 1/2" x 20"	GLB	
B-2	8 1/2" x 27 1/2"	GLB	
B-3	(3) 2x10	DF-L LUMBER	SEE 7/S701
B-4	5 1/2" x 1'-11 3/8"	GLB	

NOTES:

1. VERIFY WITH ARCH/MECH FOR HEADER LENGTH.
2. TYPICAL BOTTOM OF HEADER ELEVATION AT TOP OF OPENING. SEE ARCH/MECH FOR OPENINGS. UON ON PLAN IN PARENTHESIS AS TOP OF HEADER ELEVATION.

KEYNOTE LEGEND:

△	< < < INDICATES KEYNOTE ON PLAN
01	20" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 19.2" O/C.
02	3/4" TONGUE AND GROOVE APA RATED (SPAN RATING 48/24) STURD-I-FLOOR SHEATHING (BLOCKED). SEE S001 FOR FASTENING REQUIREMENTS.
03	PRE-ENGINEERED FLOOR TRUSS GIRDER
04	MECH OPENING - VERIFY LOCATION AND EXACT SIZE WITH MECH. FASTEN SHEATHING WITH EDGE NAILING PATTERN AND BLOCKING SUPPORTED BY ADJACENT TRUSSES AROUND OPENING. TRUSS SUPPLIER ADD TRUSS AS NEEDED TO MAINTAIN SPACING
05	1 3/4" x 18" LVL STAIR STRINGER
06	6 1/2" STRUCTURAL INSULATED PANEL WALL
07	SIMPSON HDUE AT COLUMN ABOVE AND BELOW FLOOR. SEE COLUMN SCHEDULE FOR SIZE
08	(2) 1/2"x20"x20" GALV SHEAR TRANSFER PLATES W/ (5) 1/2"Ø THROUGH BOLTS AND (14) SIMPSON SDS25200 -SEE 7/S701
09	SIP HEADER DESIGN BY SIP SUPPLIER
10	2x10 DFL No. 2 FRAMING 16" O/C AT LANDING
11	(2) 2x10 DFL No. 2 HEADER AT LANDING
12	2x10 LEDGER AT WALL FOR LANDING SUPPORT. ATTACHMENT BY SIP SUPPLIER FOR MIN 300 PLF

NOTES:
ROOF DIAPHRAGM LATERAL FORCES GIVEN FOR SIP
SHEAR WALL DESIGN AS STRENGTH LEVEL.
←-XXk



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PROJECT DESCRIPTION
NEW TOWN FOOTBALL FIELD IMPROVEMENTS

CITY WATFORD CITY
STATE ND

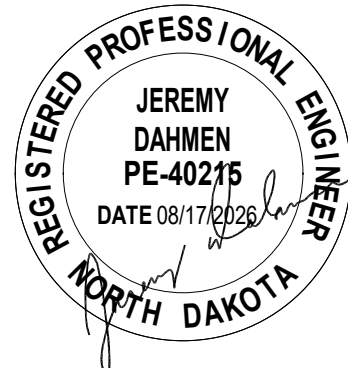
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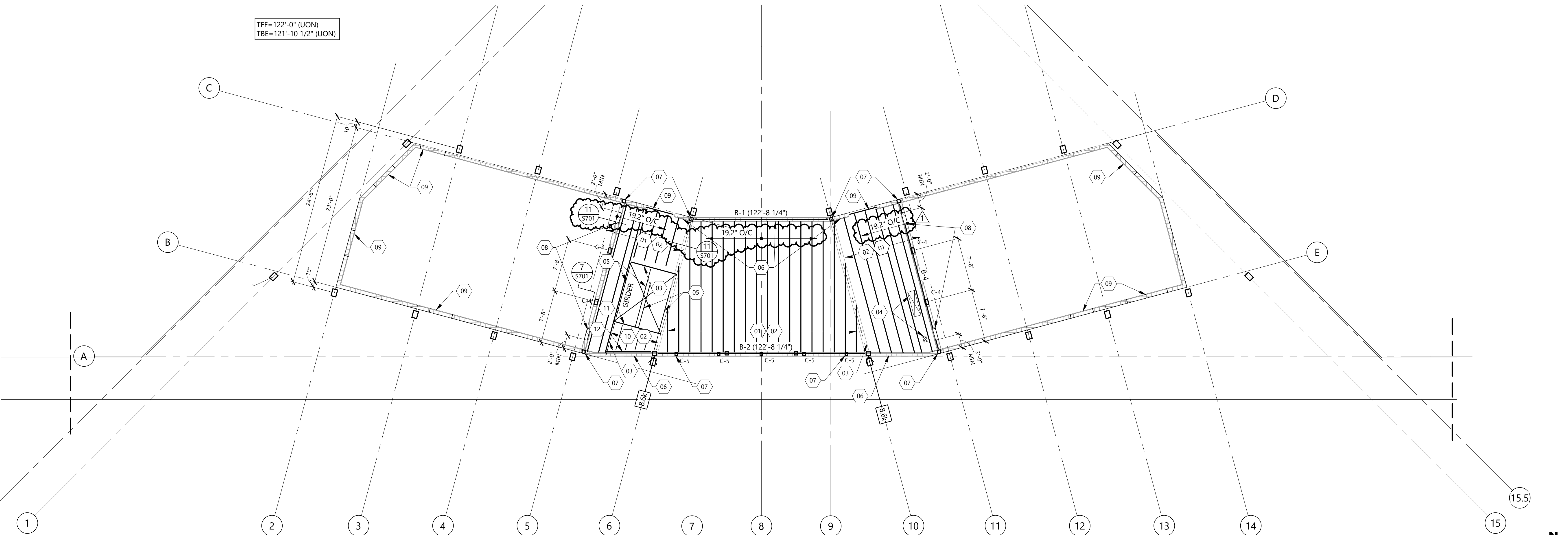
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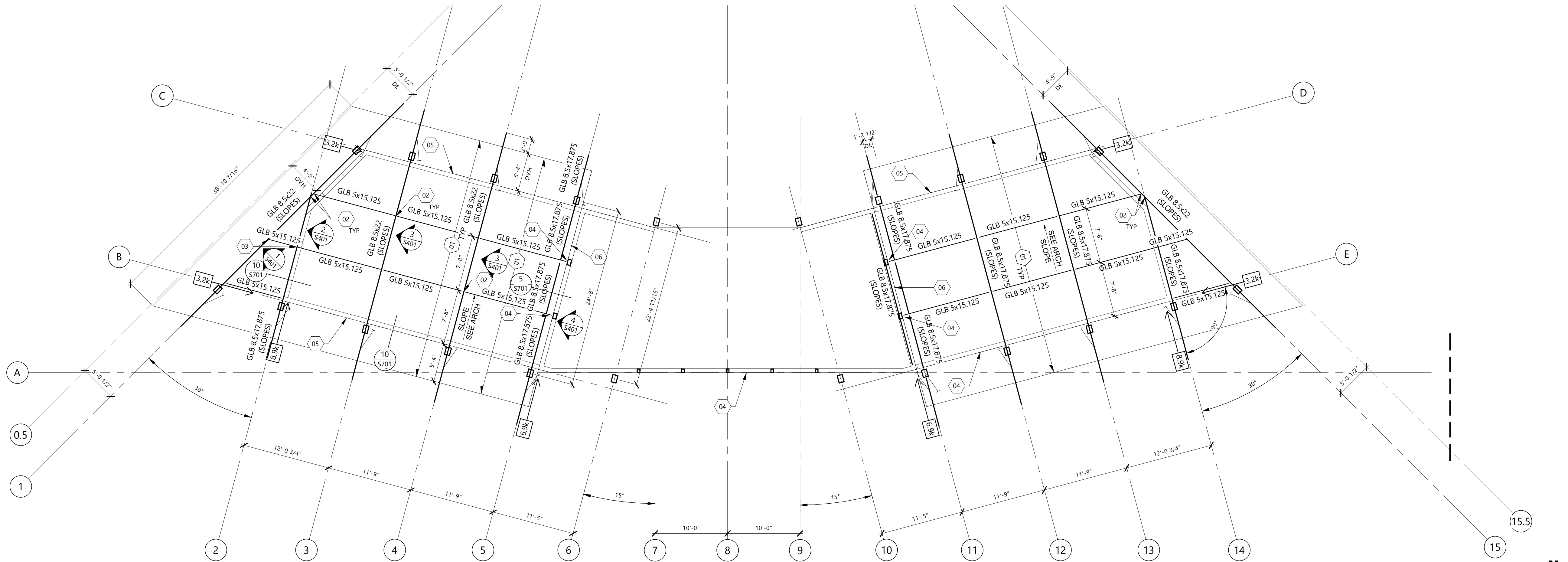
DRAWING TITLE
PRESS BOX FLOOR FRAMING PLAN

S203

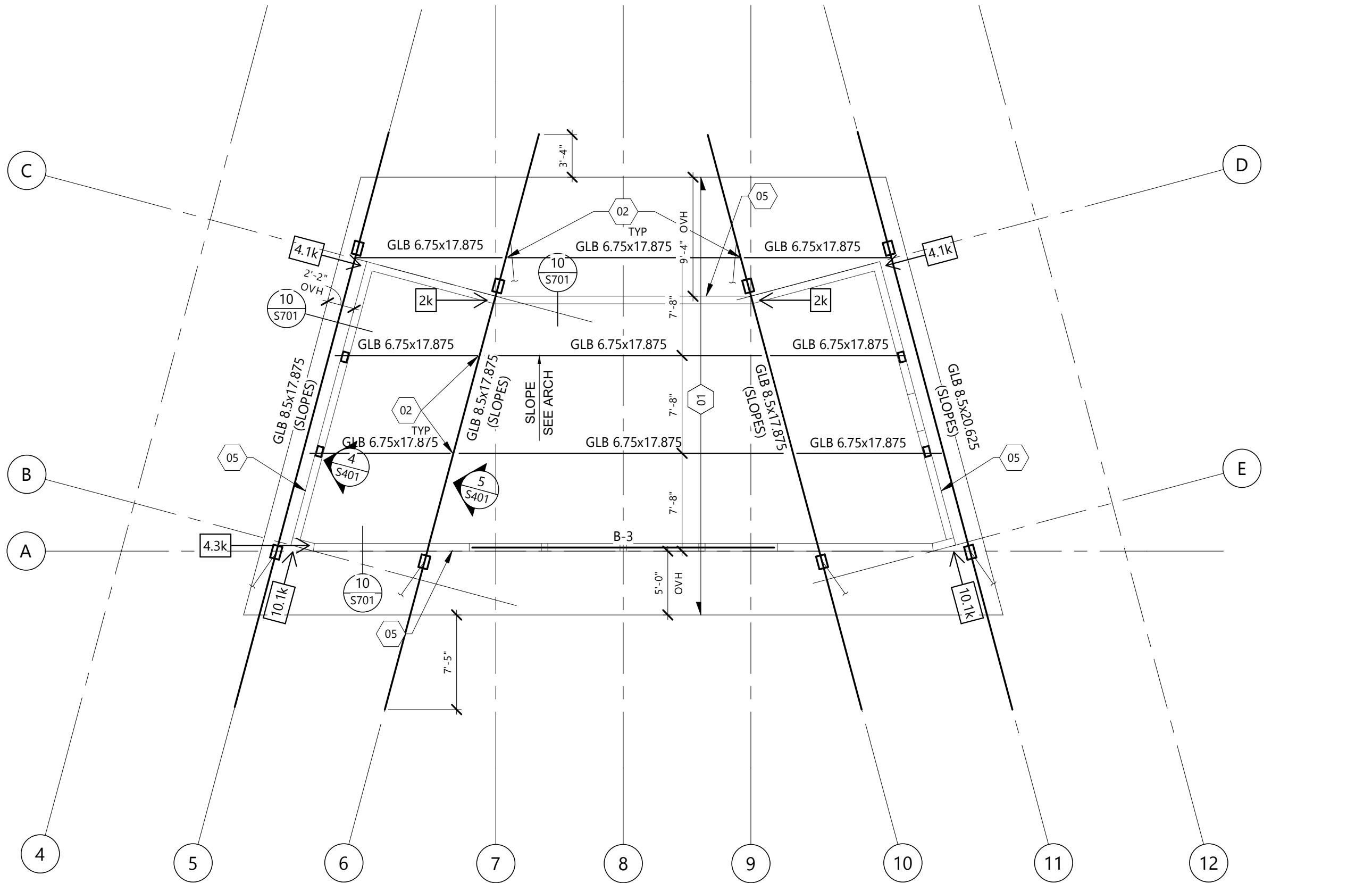


1 PRESS BOX FLOOR FRAMING PLAN
S203 1/8" = 1'-0"

1
S204
ROOF FRAMING PLAN
1/8" = 1'-0"



2
S204
HIGH ROOF FRAMING
1/8" = 1'-0"



WOOD BEAM SCHEDULE			
MARK	SIZE	MATERIAL	REMARKS
B-1	5 1/2" x 20"	GLB	
B-2	8 1/2" x 27 1/2"	GLB	
B-3	(3) 2x10	DF-L LUMBER	SEE 7/S701
B-4	5 1/2" x 1'-11 3/8"	GLB	

- NOTES:
1. VERIFY WITH ARCH/MECH FOR HEADER LENGTH.
 2. TYPICAL BOTTOM OF HEADER ELEVATION AT TOP OF OPENING, SEE ARCH/MECH FOR OPENINGS. UON ON PLAN IN PARENTHESIS AS TOP OF HEADER ELEVATION.

KEYNOTE LEGEND:	
01	10 1/4" STRUCTURAL INSULATED ROOF PANEL L-SPLINE.
02	SIMPSON CBH (GALVANIZED) AT BEAM TO GLULAM COLUMN & BEAM TO BEAM CONNECTIONS.
03	NOT USED
04	INVERTED FLANGE FACE MOUNT HANGER BY MASS TIMBER SUPPLIER.
05	6 1/2" STRUCTURAL INSULATED WALL PANEL.
06	2x10 LEDGER AT WALL FOR LOWER ROOF SIP DIAPHRAGM CONNECTION. CONNECTION BY SIP SUPPLIER, SEE 5/S701.

NOTES:
DIAPHRAGM LATERAL FORCES GIVEN FOR SIP
SHEAR WALL DESIGN AS STRENGTH LEVEL.
Xk



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CITY WATFORD CITY
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DRAWING TITLE
ROOF FRAMING PLAN

S204

KEYNOTE LEGEND:

< < INDICATES KEYNOTE ON PLAN

01GLUE-LAMINATED BEAM - SEE PLAN.

02GLUE-LAMINATED COLUMN - SEE PLAN.

03GLUE-LAMINATED JOIST.

04GLUE-LAMINATED BRACE.

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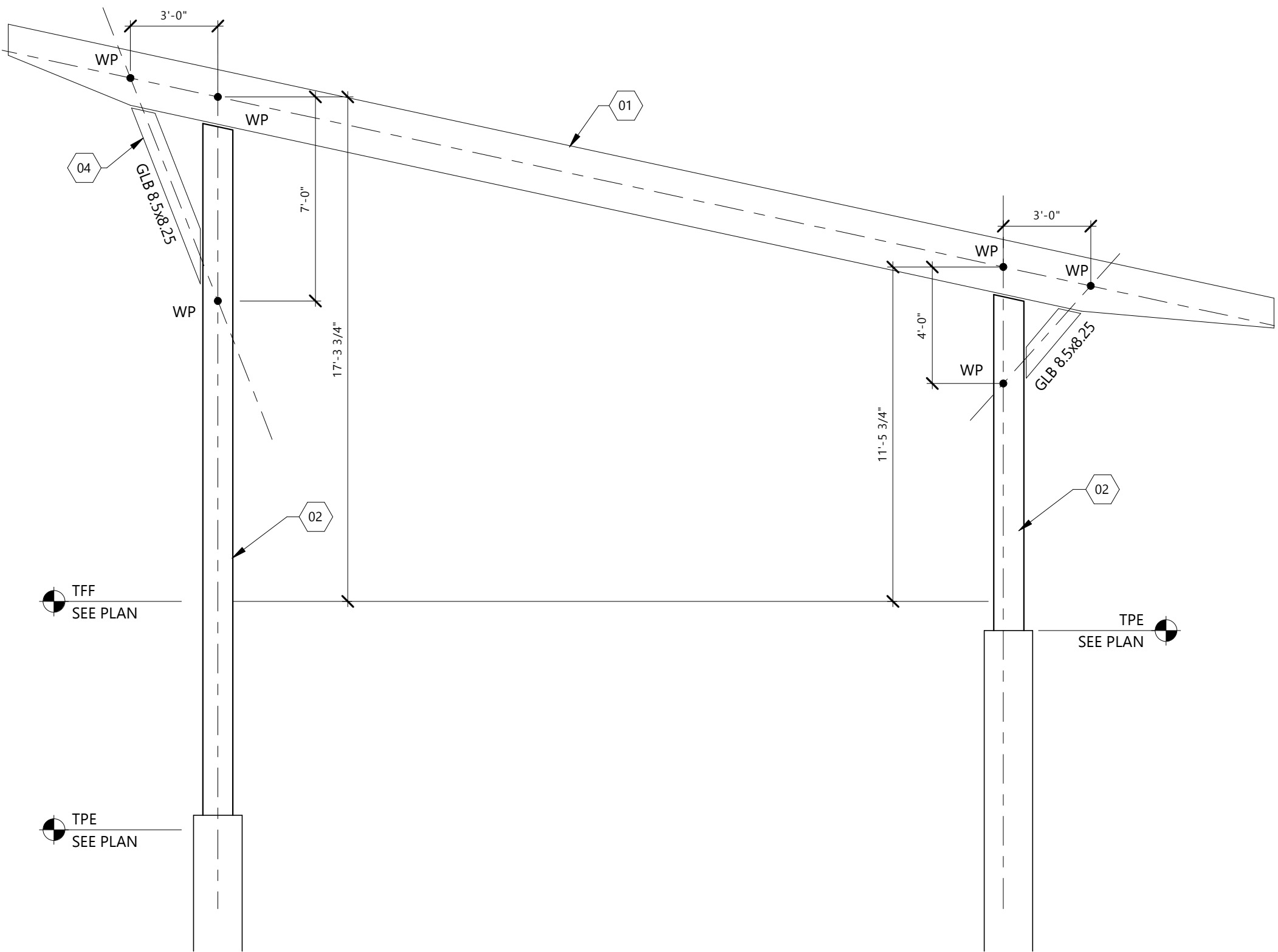
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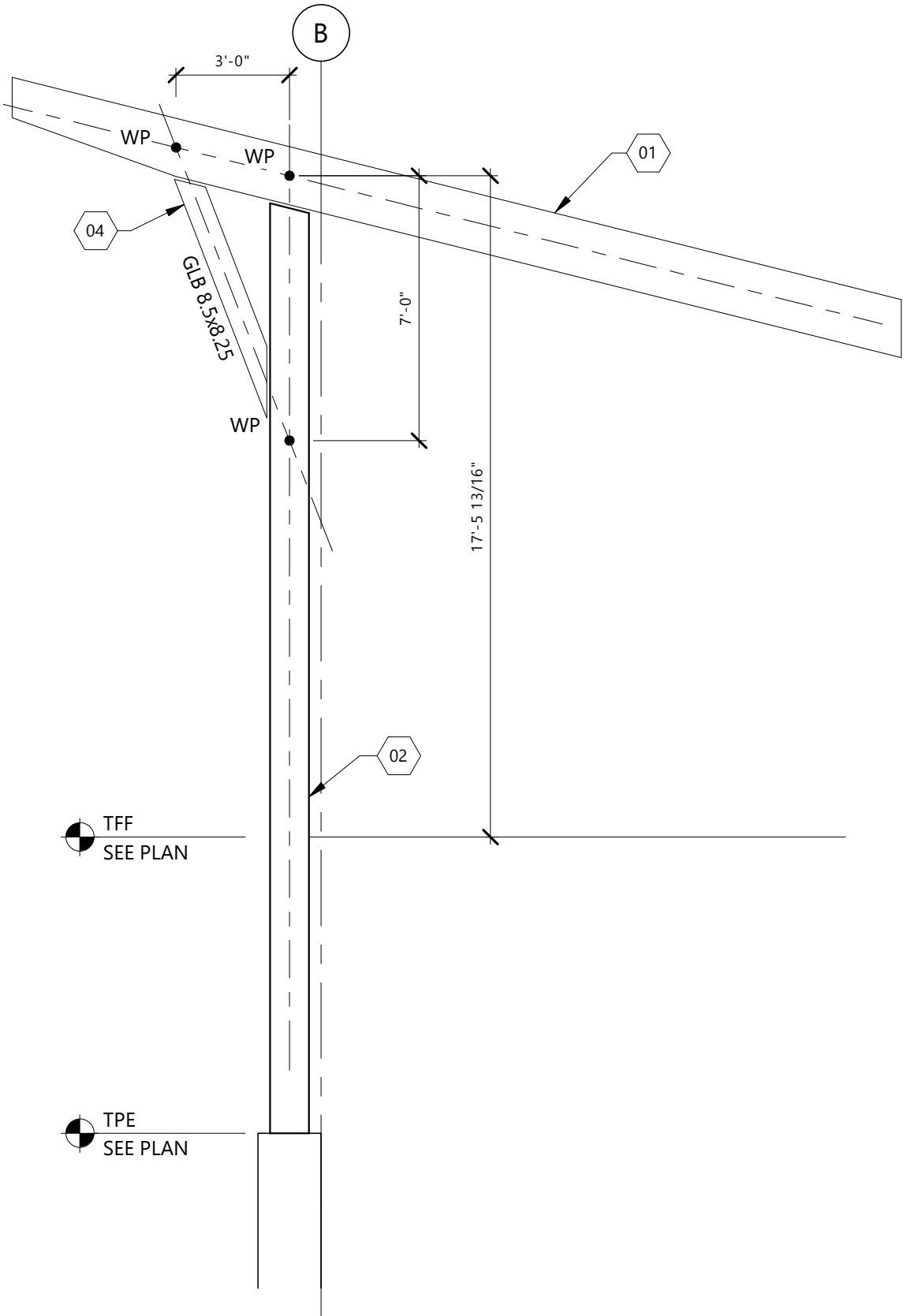


DRAWING TITLE
FRAMING ELEVATIONS

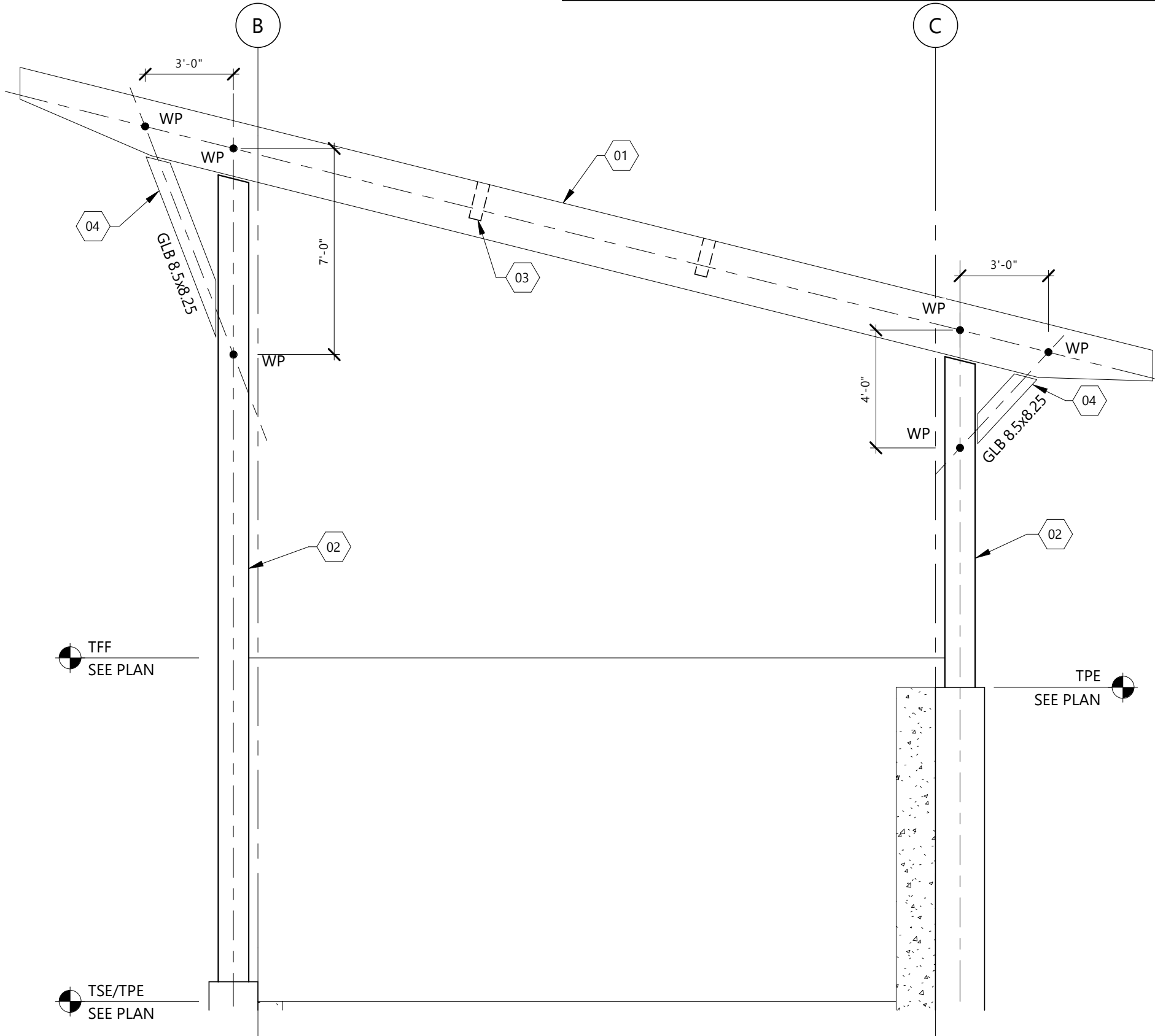
S401



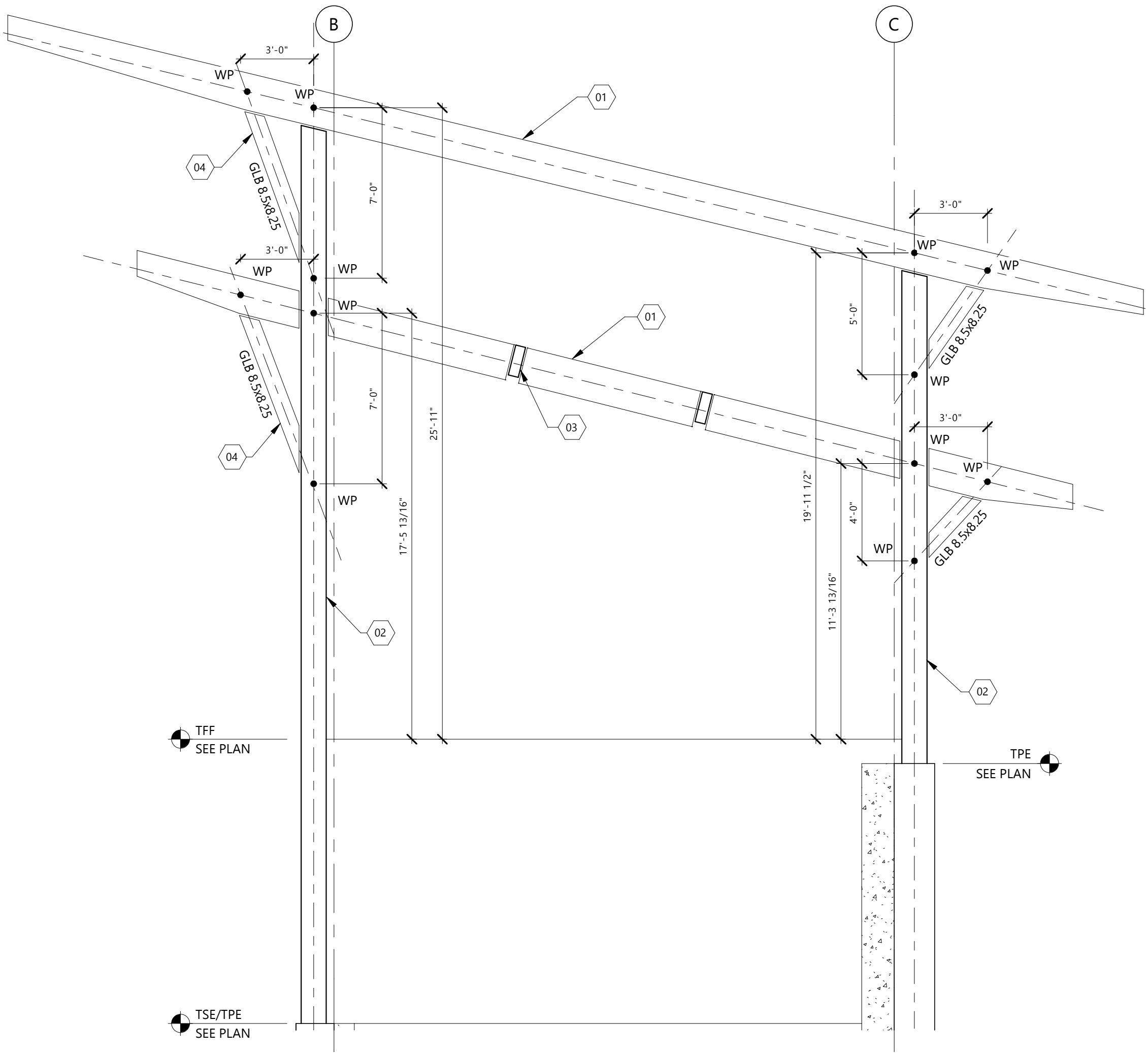
1 FRAMING ELEVATION @ GRIDS 1 & 15
S401 1/4" = 1'-0"



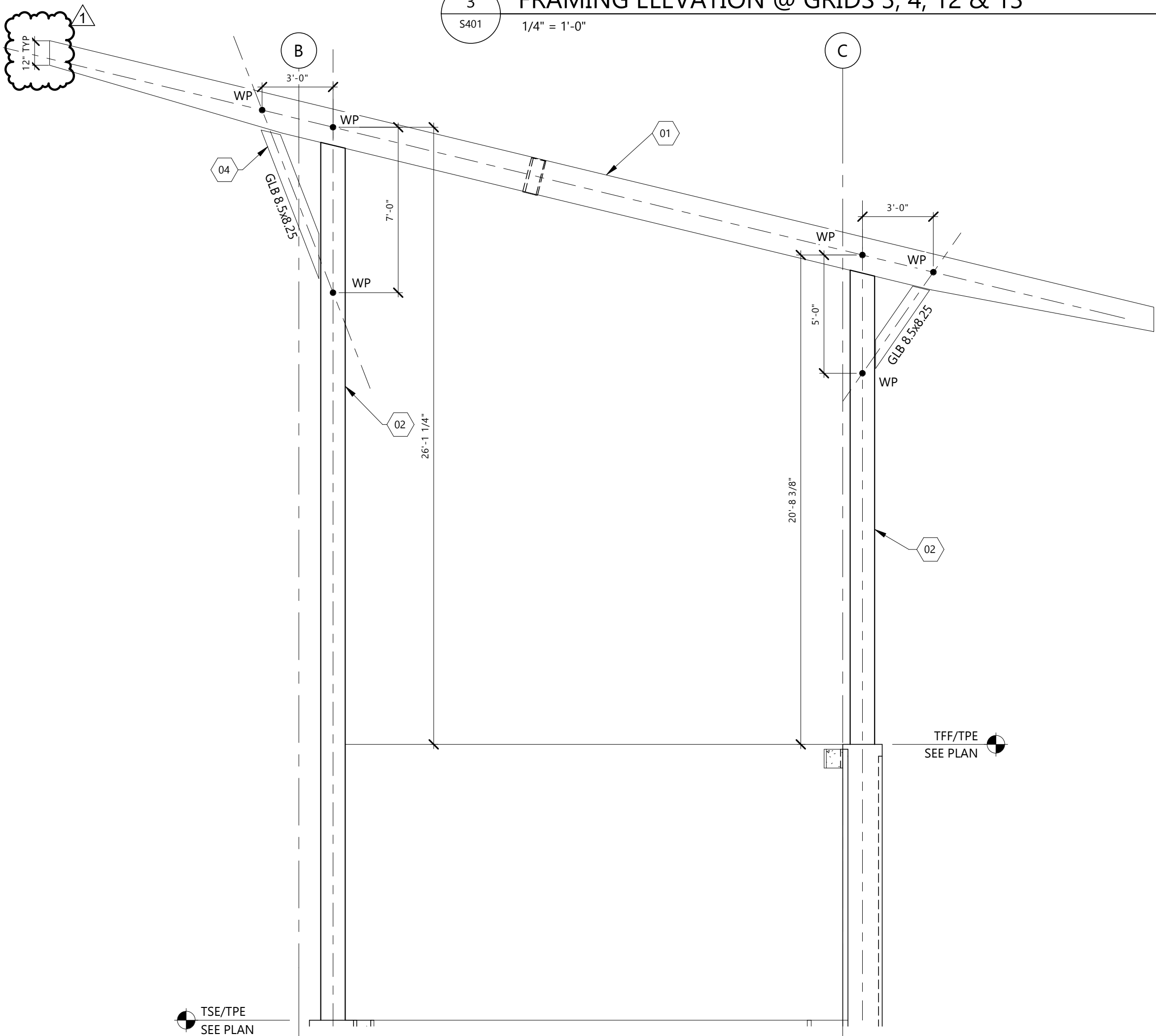
2 FRAMING ELEVATION @ GRIDS 2 & 14
S401 1/4" = 1'-0"



3 FRAMING ELEVATION @ GRIDS 3, 4, 12 & 13
S401 1/4" = 1'-0"



4 FRAMING ELEVATION @ GRIDS 5 & 11
S401 1/4" = 1'-0"



5 FRAMING ELEVATION @ GRIDS 6 & 10
S401 1/4" = 1'-0"

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PROJECT DESCRIPTION
NEW TOWN FOOTBALL
FIELD IMPROVEMENTS

CITY WATFORD CITY
STATE ND

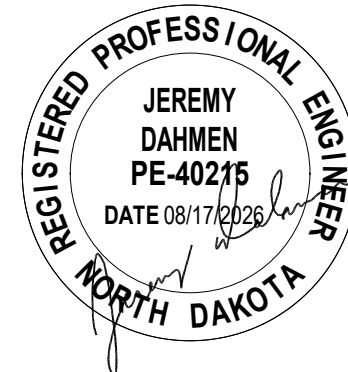
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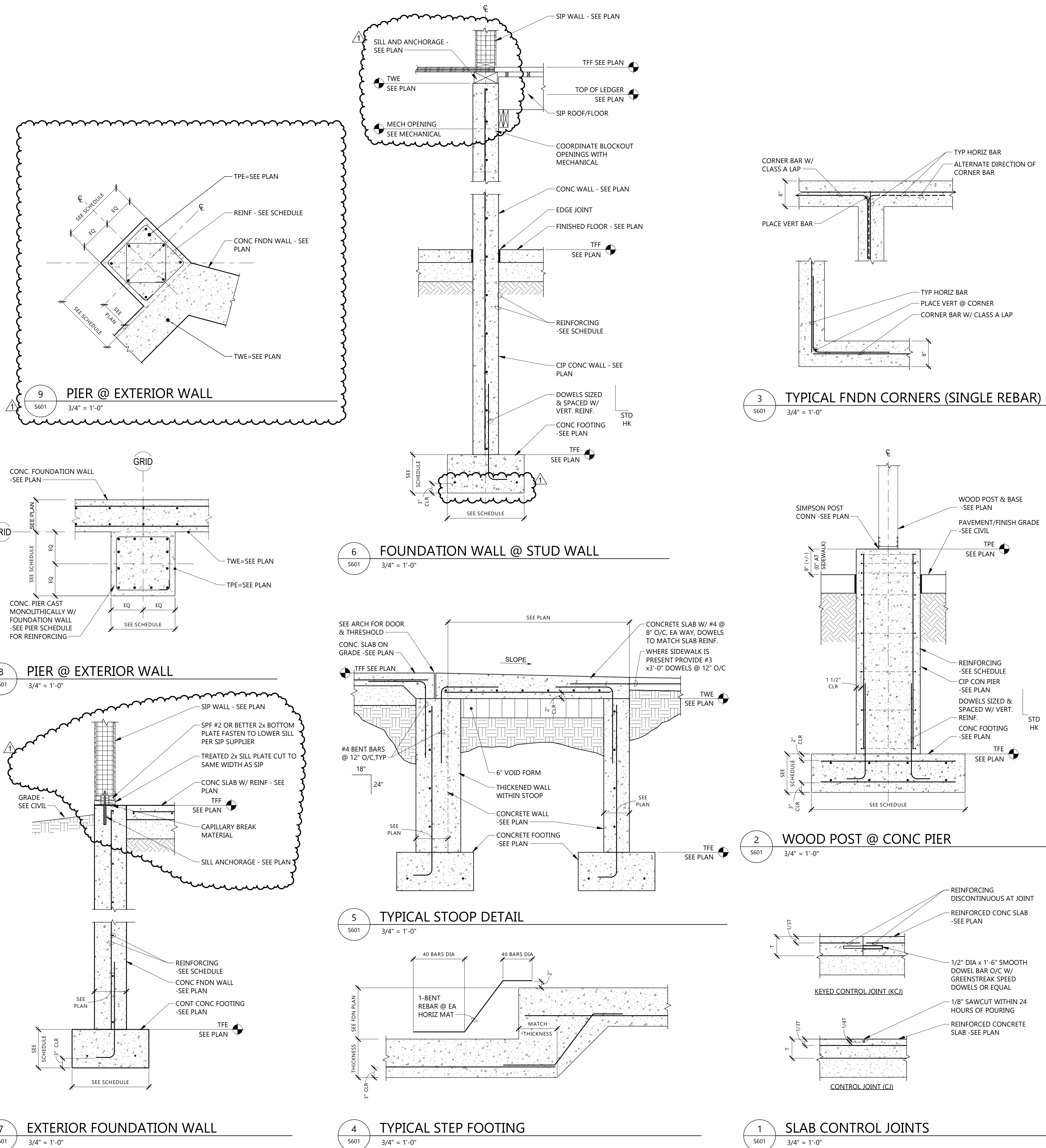
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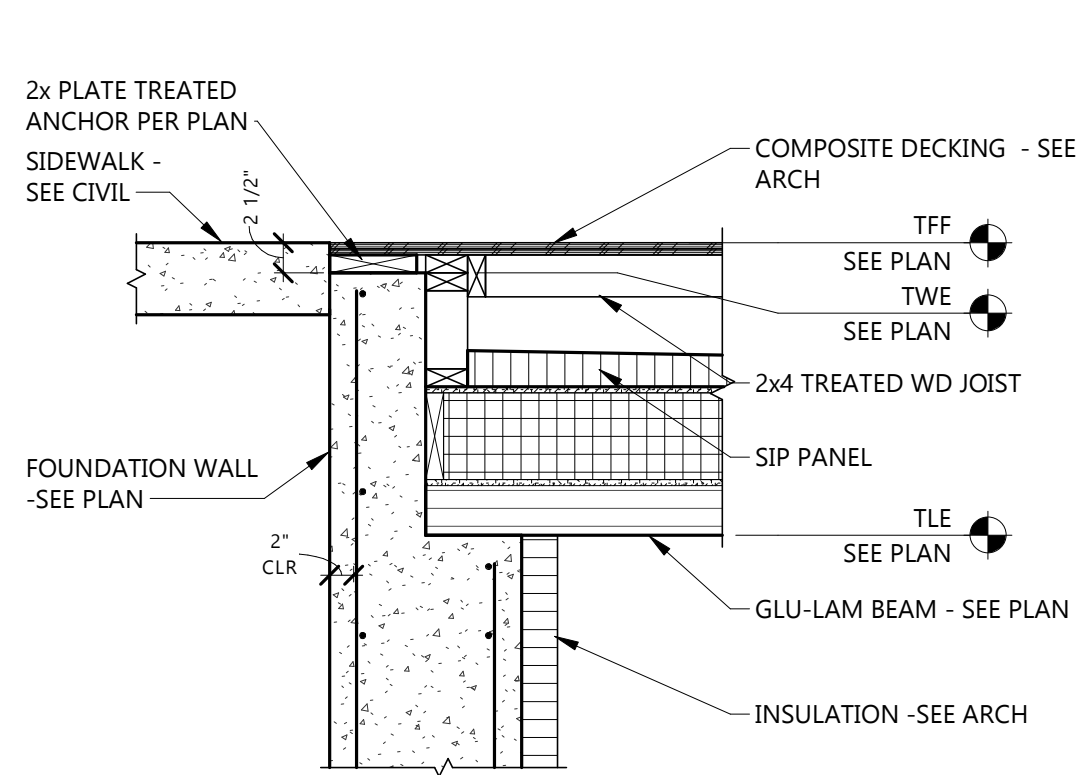


DRAWING TITLE
FOUNDATION DETAILS

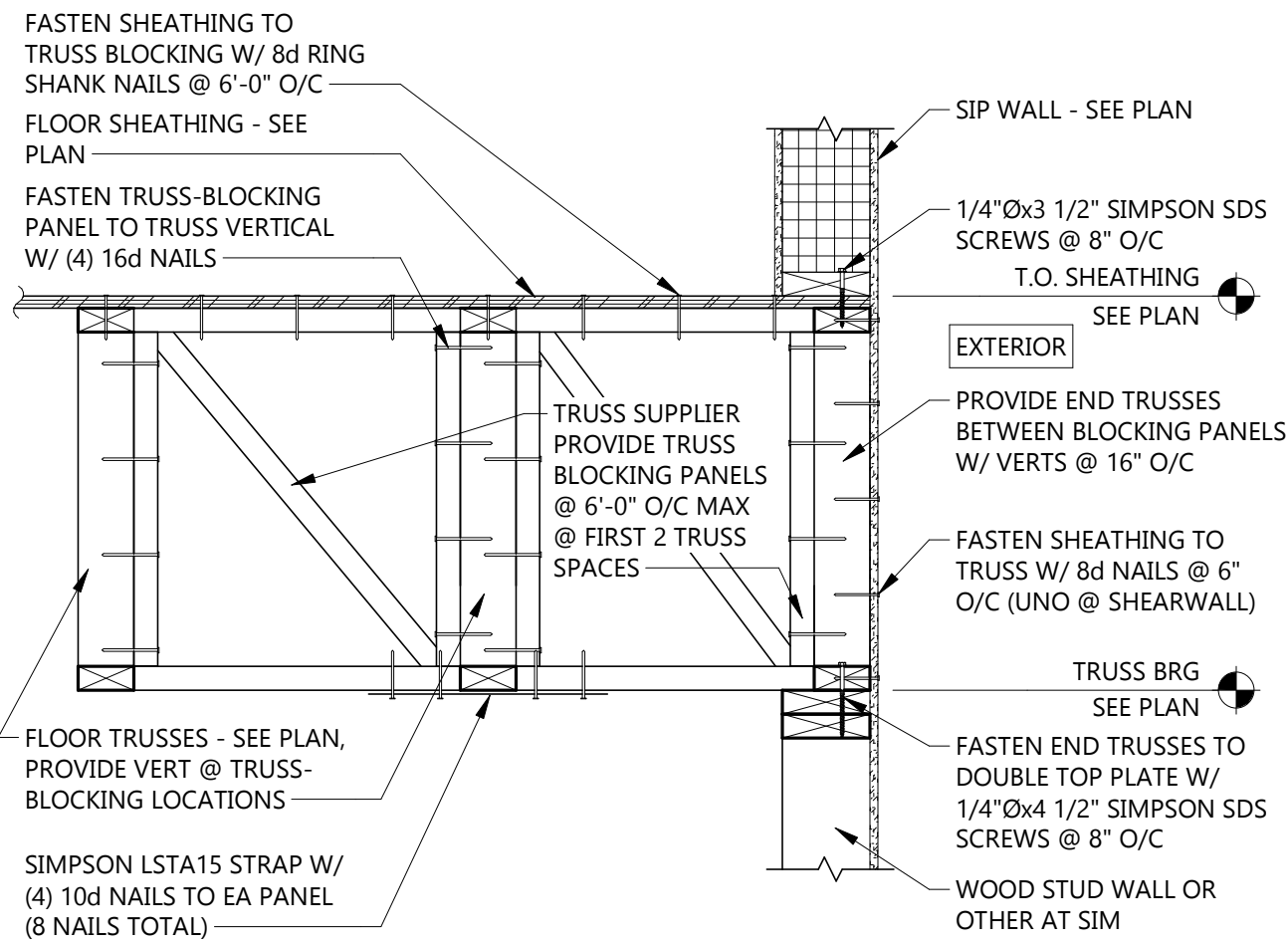
S601



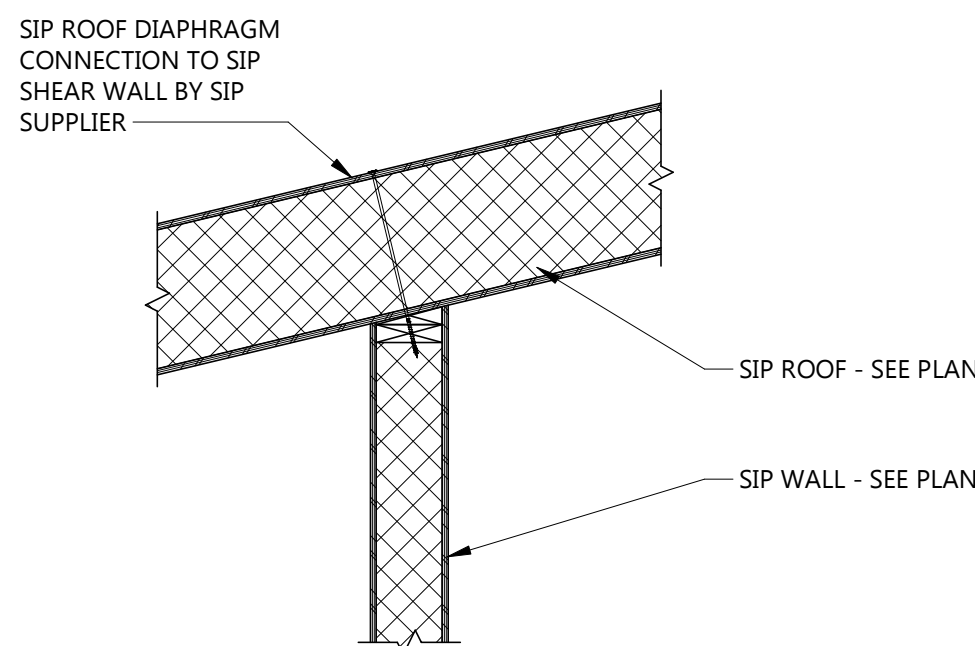




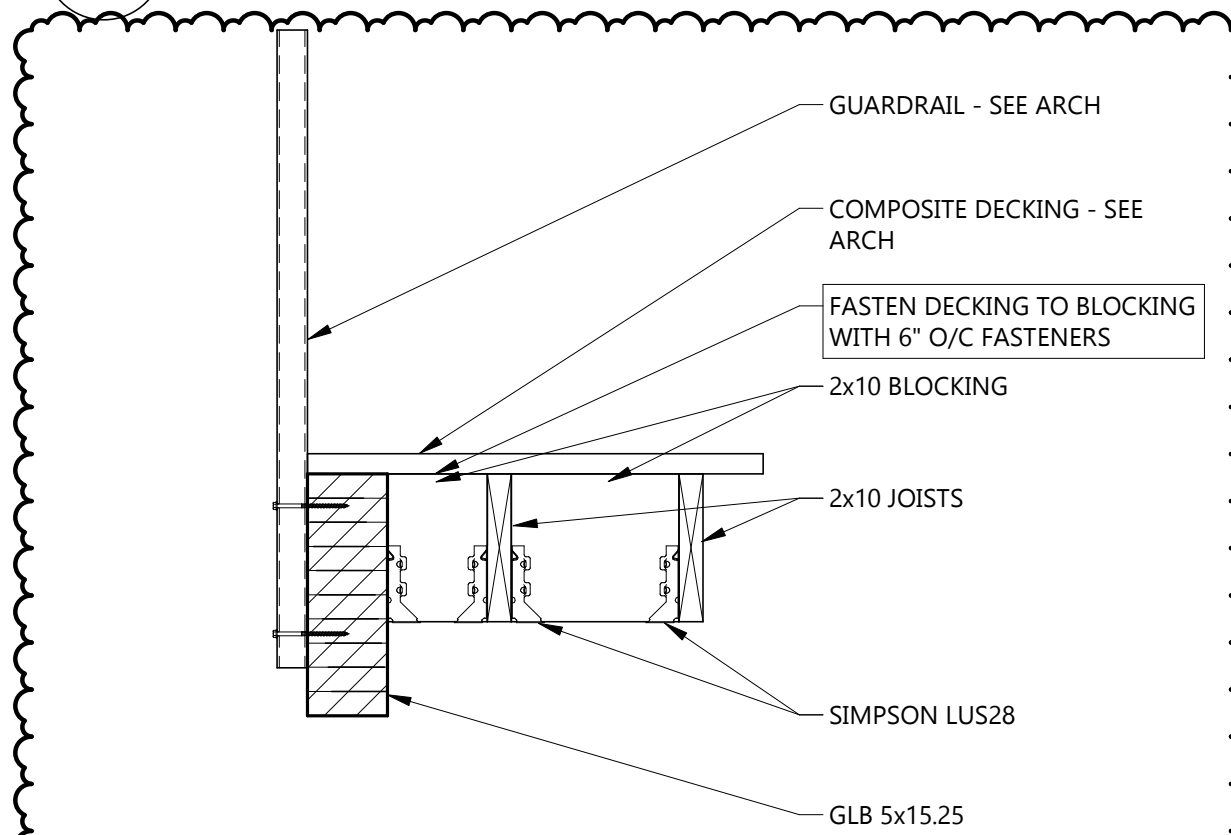
13 NORTH FOUNDATION WALL DETAIL
S701 3/4" = 1'-0"



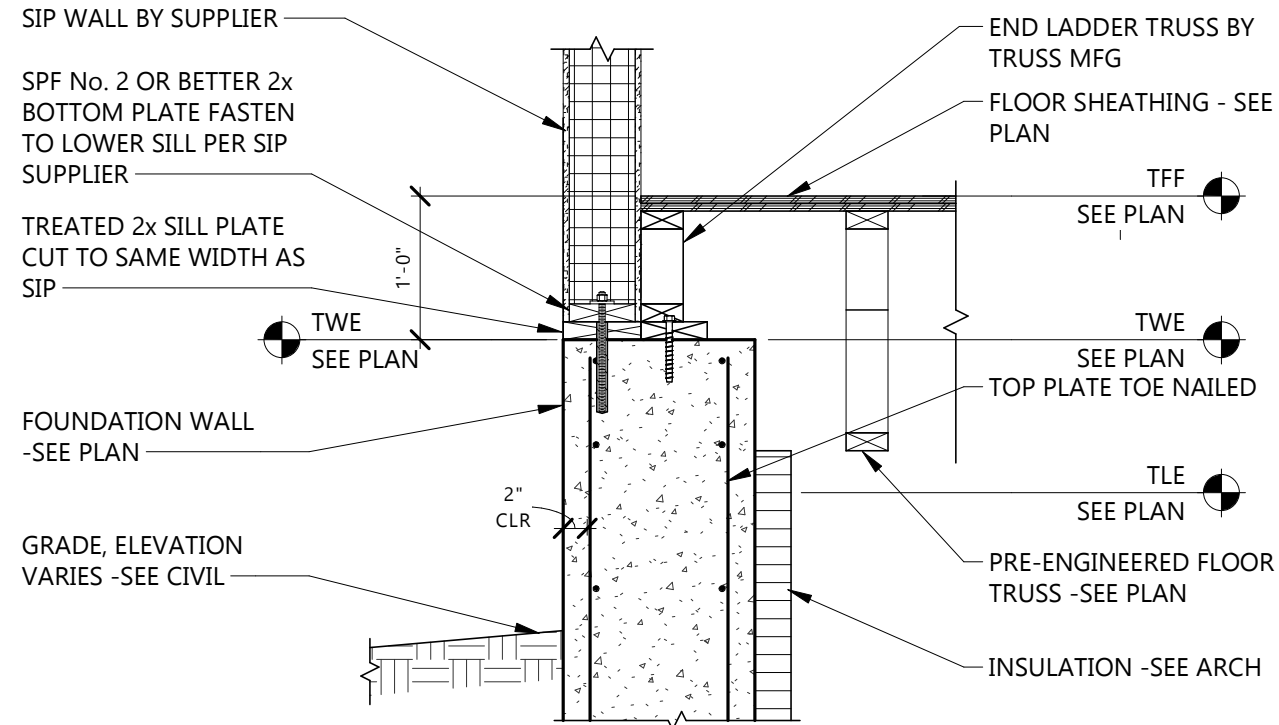
11 TYPICAL END WALL DETAIL
S701 1" = 1'-0"



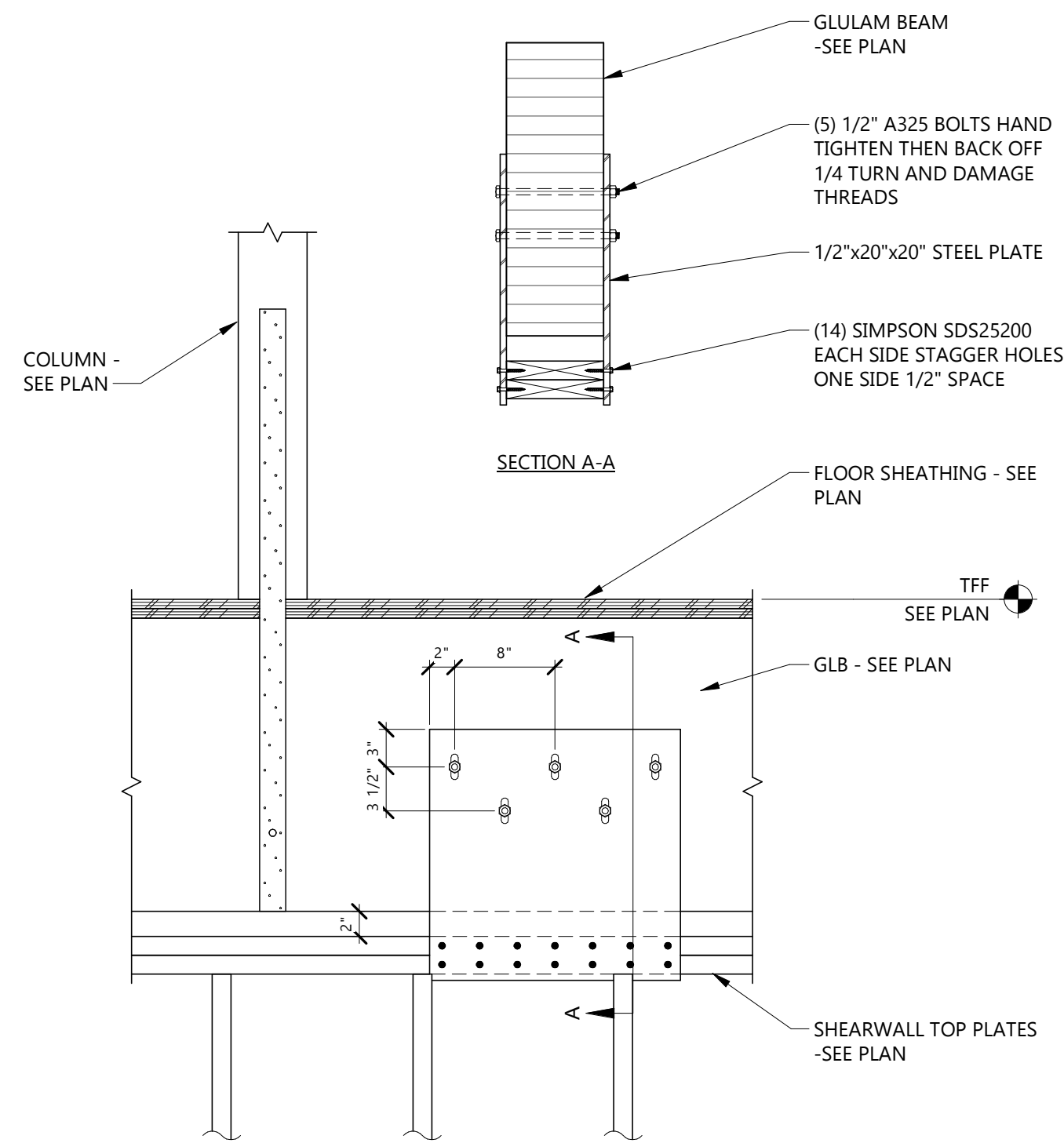
10 SIP WALL TO SIP ROOF PANEL CONNECTION
S701 3/4" = 1'-0"



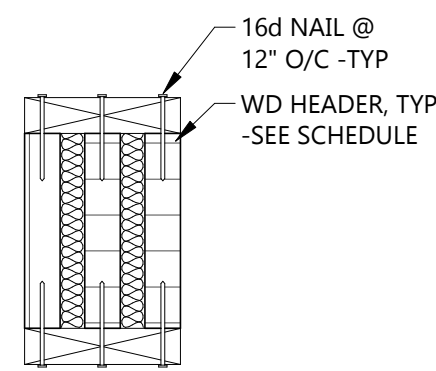
9 JOIST BLOCKING
S701 1" = 1'-0"



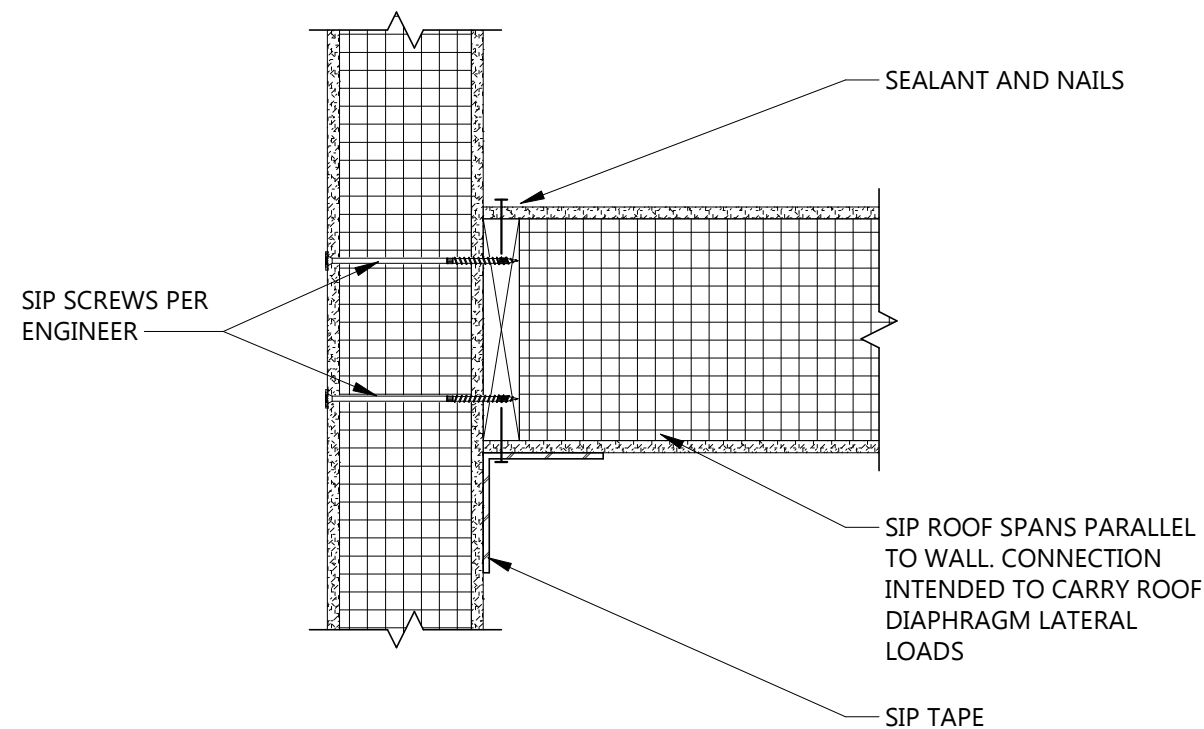
8 FLOOR TRUSS LAP DETAIL
S701 3/4" = 1'-0"



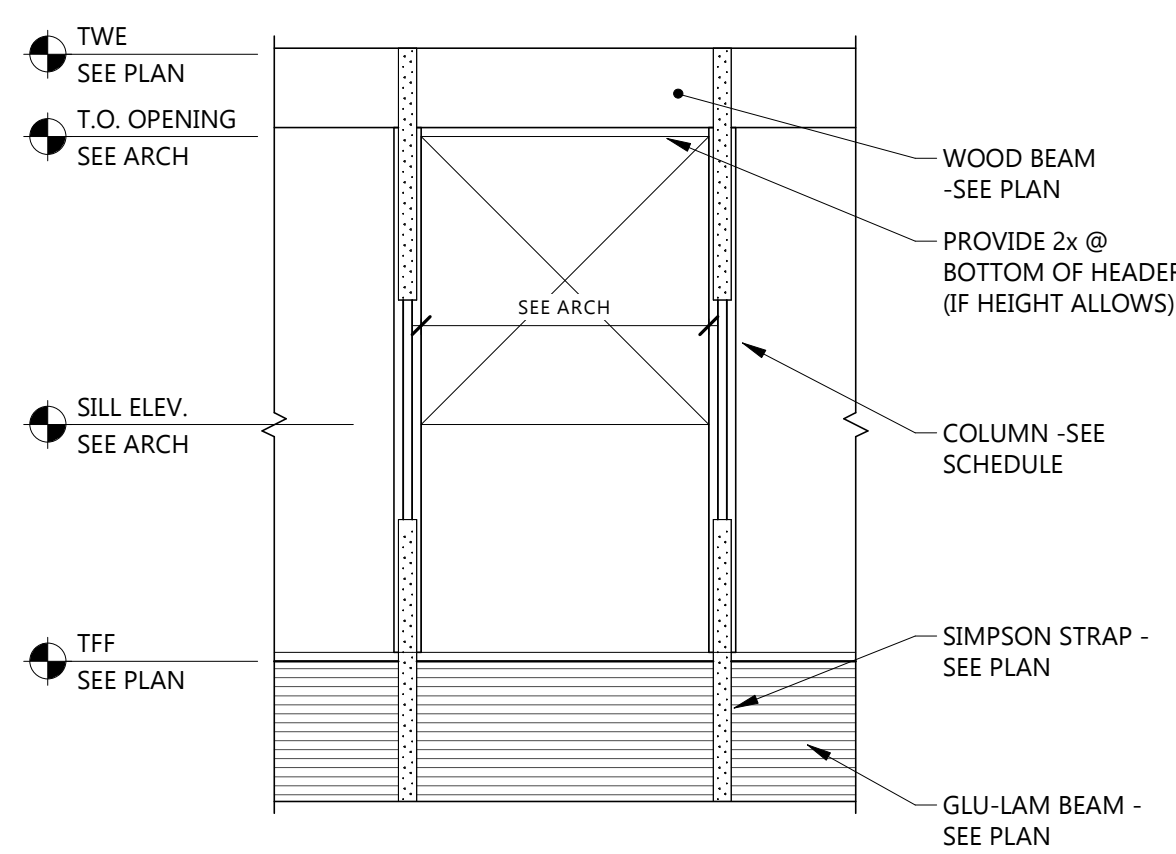
7 GLULAM CONN @ TOP PLATE
S701 1" = 1'-0"



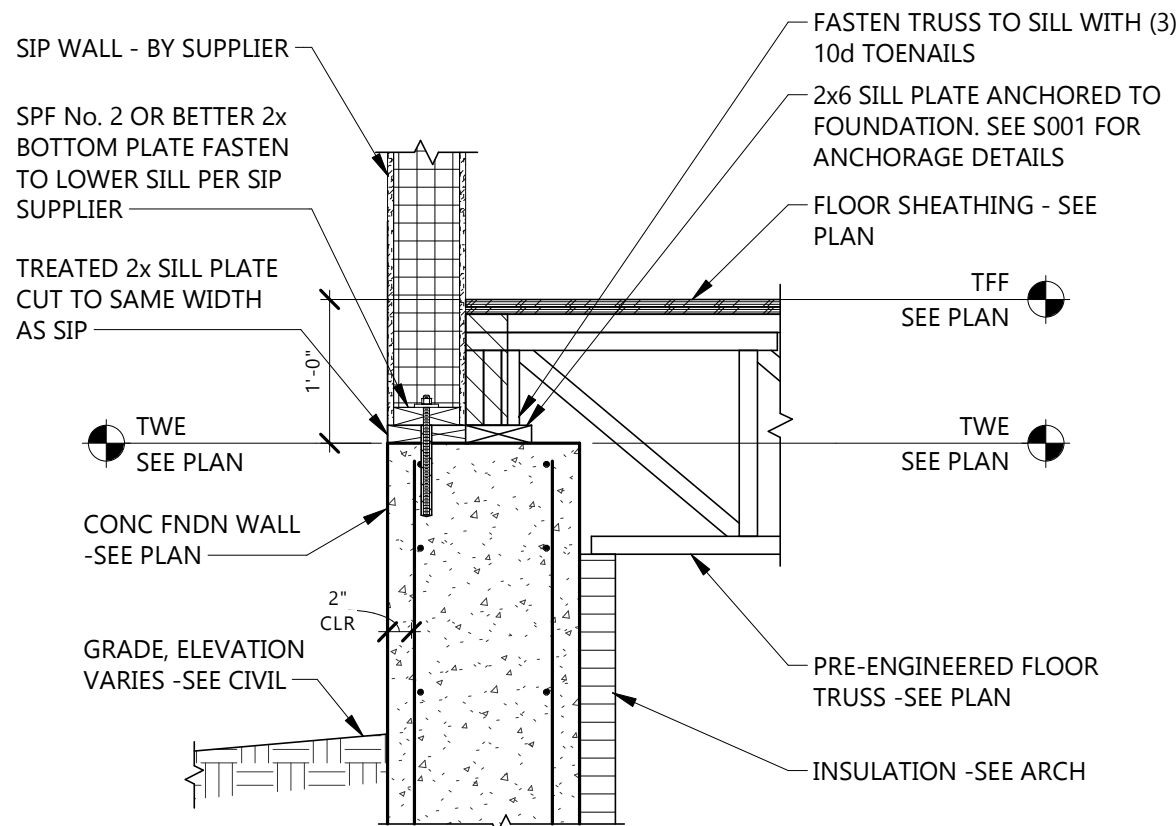
6 TYP WOOD HEADER DETAIL
S701 1 1/2" = 1'-0"



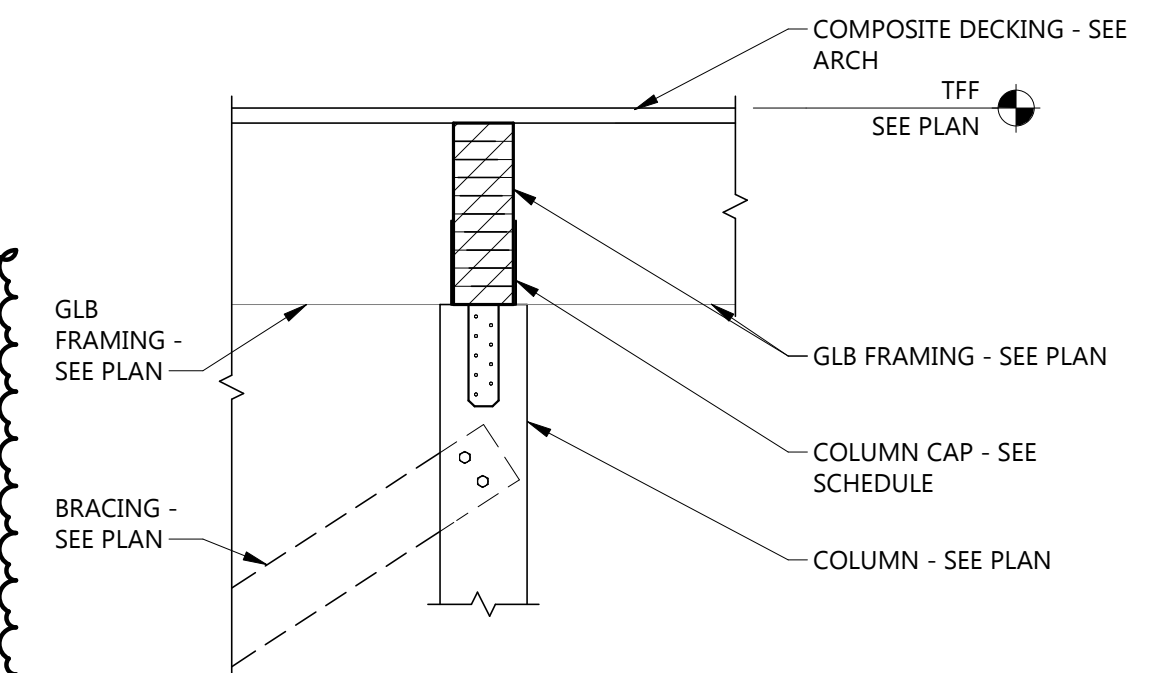
5 CONN @ SIP ROOF
S701 1 1/2" = 1'-0"



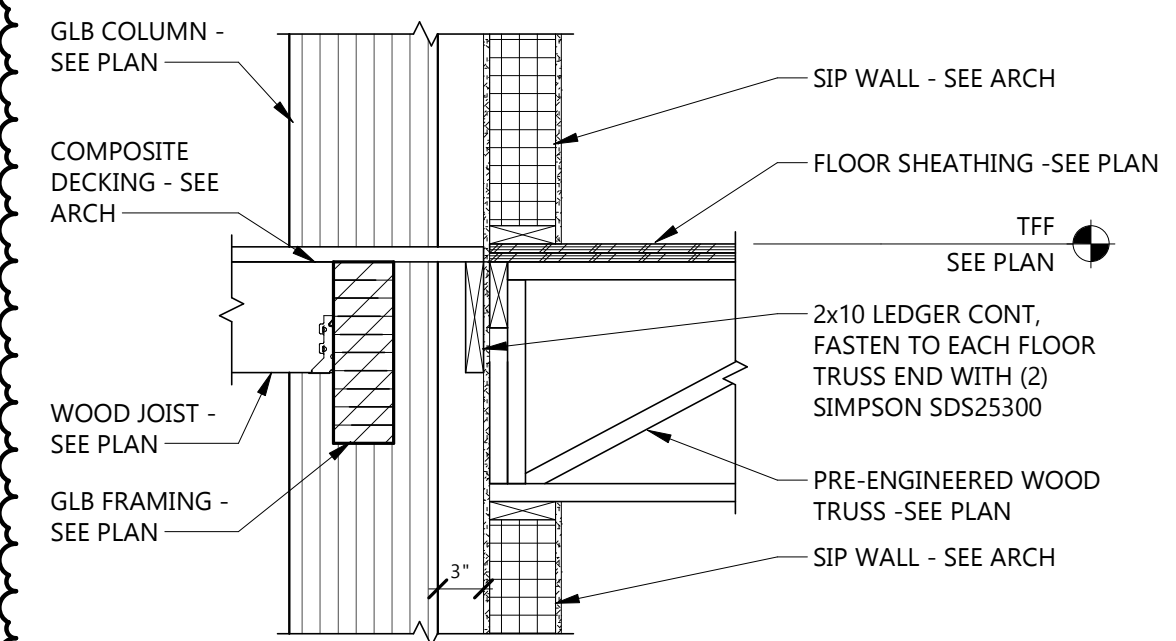
4 FRAMED OPENING IN SIP WALL
S701 3/8" = 1'-0"



3 FLOOR TRUSS BEARING DETAIL
S701 3/4" = 1'-0"



2 FRAMING @ DECK
S701 3/4" = 1'-0"



1 EXTERIOR WALL @ GRANDSTAND DECK
S701 3/4" = 1'-0"

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NEW TOWN SCHOOL DISTRICT

PROJECT DESCRIPTION
NEW TOWN FOOTBALL FIELD IMPROVEMENTS

CITY WATFORD CITY
STATE ND

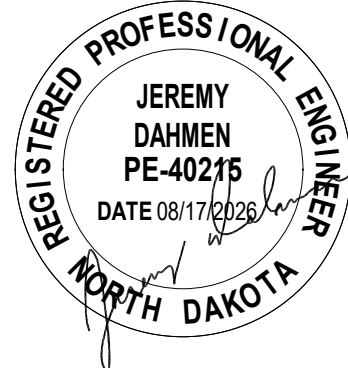
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FRAMING DETAILS

S701

1	ADDENDUM 01	08/26/2026
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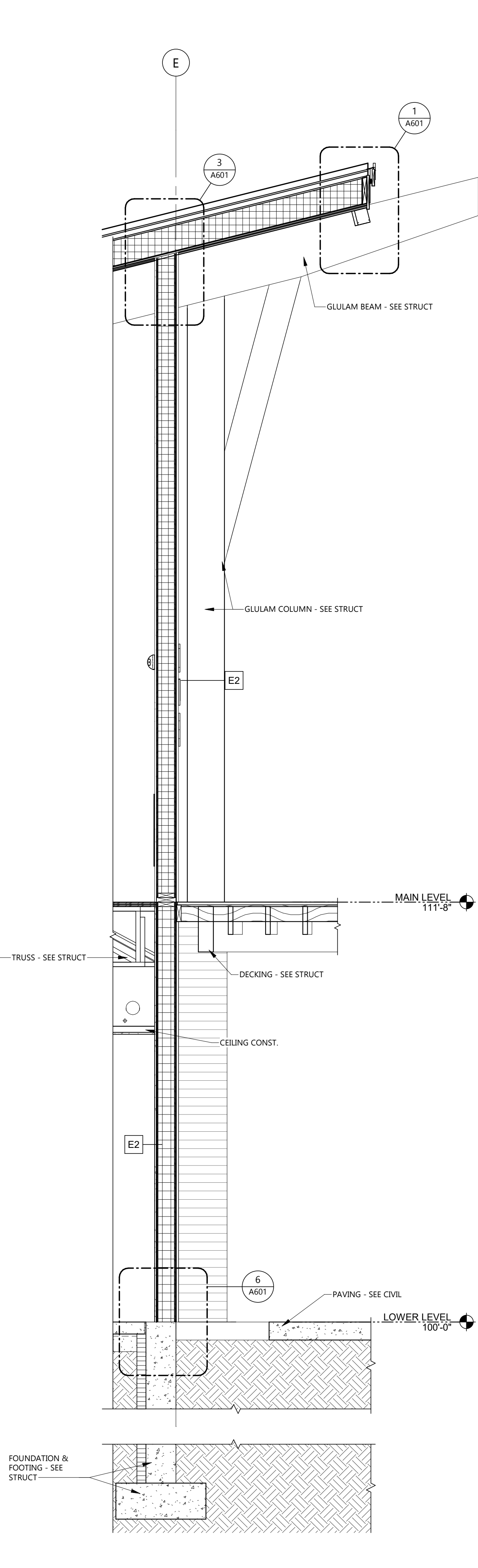
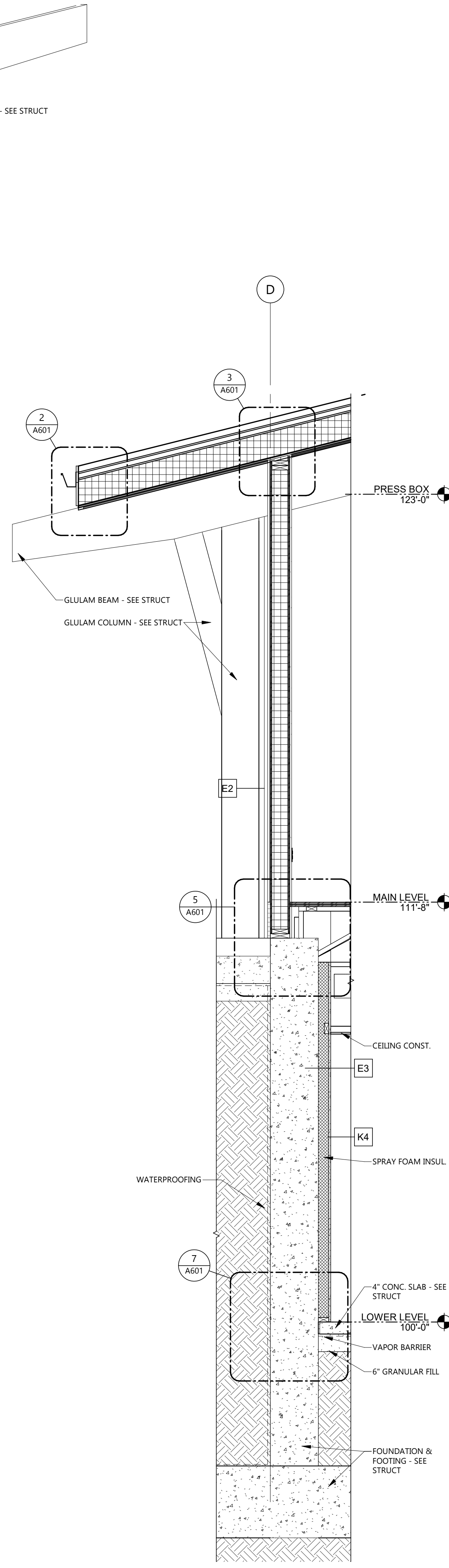
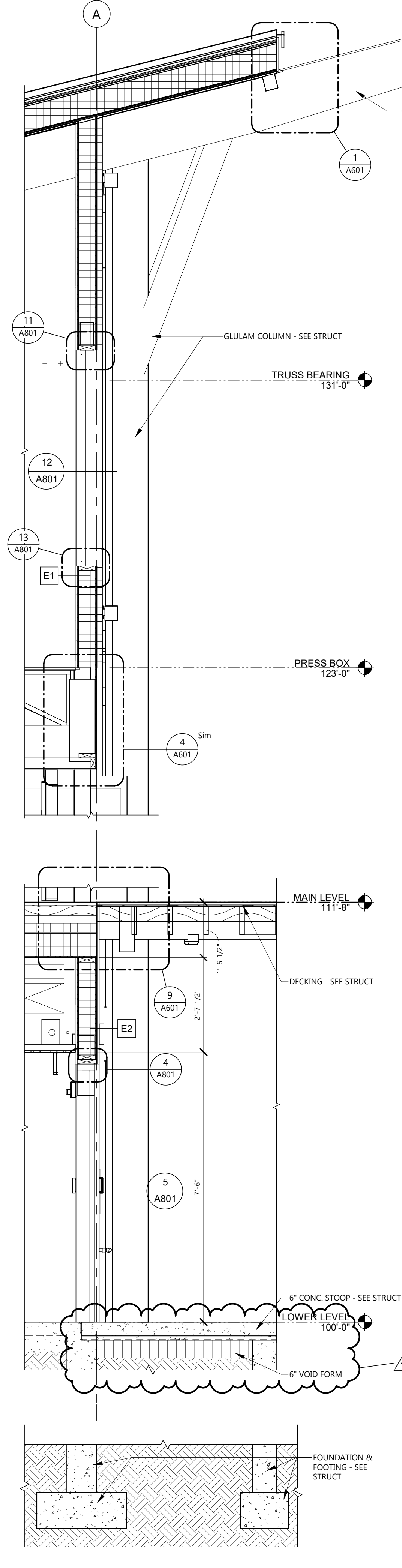
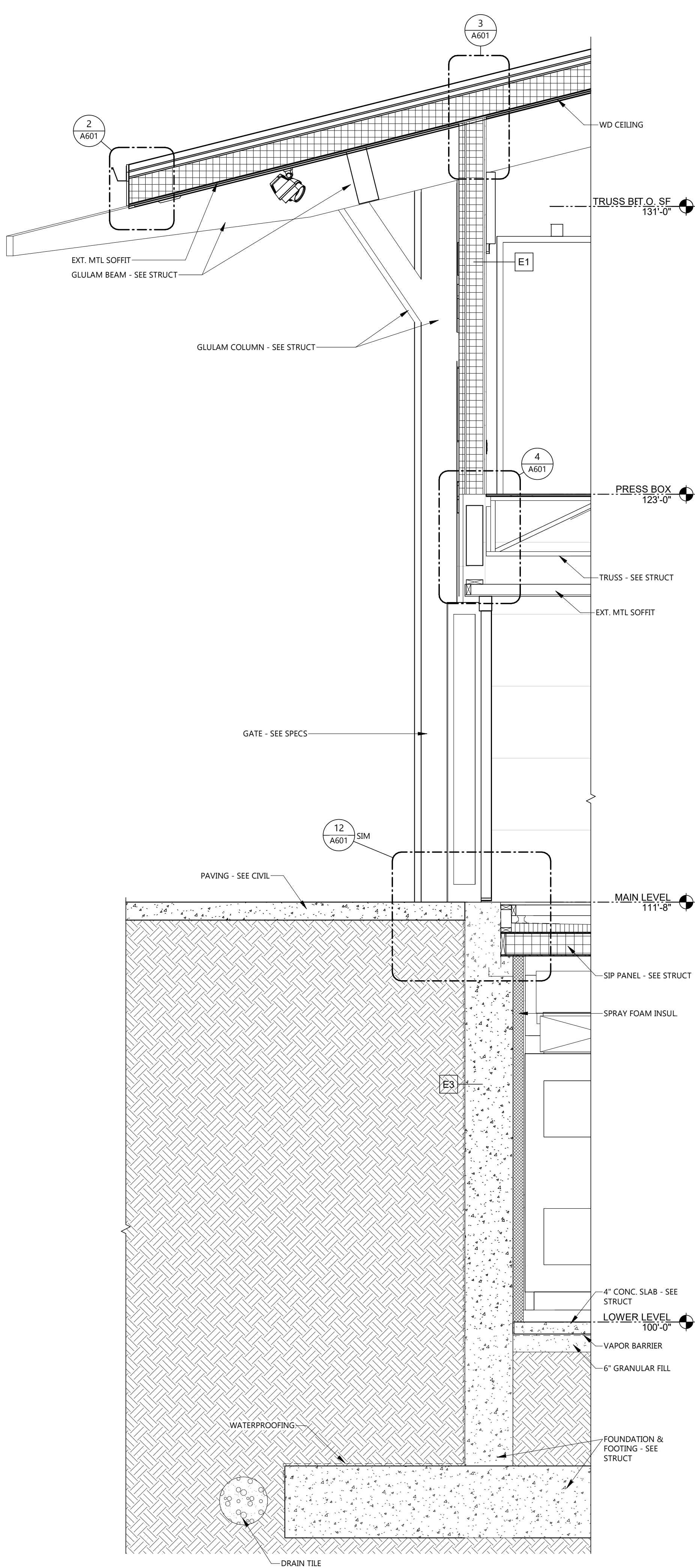
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DRAWING TITLE
WALL SECTIONS

A441



1	ADDENDUM 01	08/26/2026
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MARK	DESCRIPTION	DATE

PROJECT NO: 20263400

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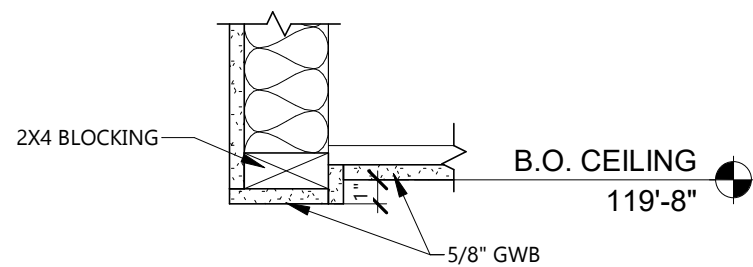
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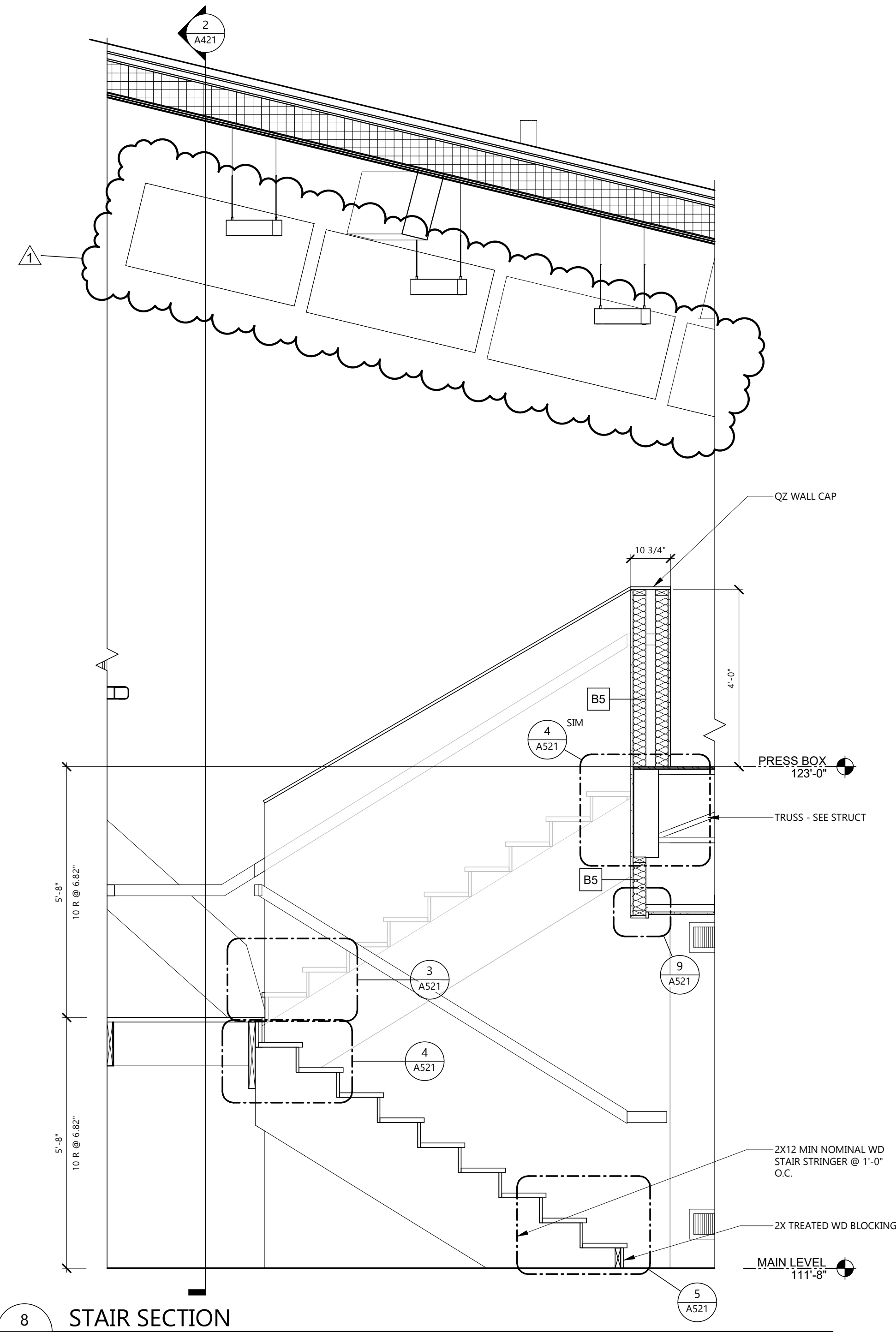
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STAIR PLANS AND
SECTIONS, DETAILS

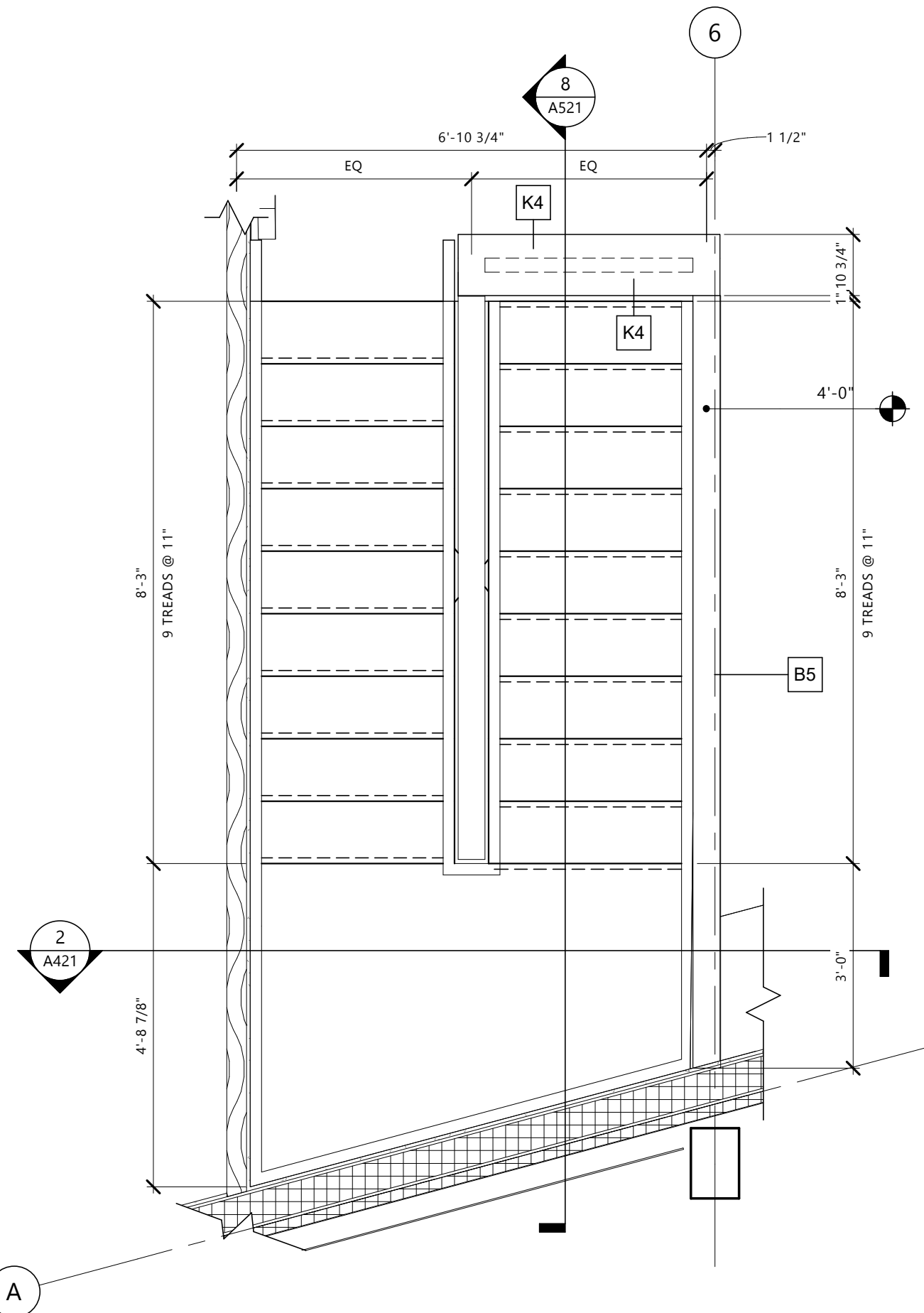
A521



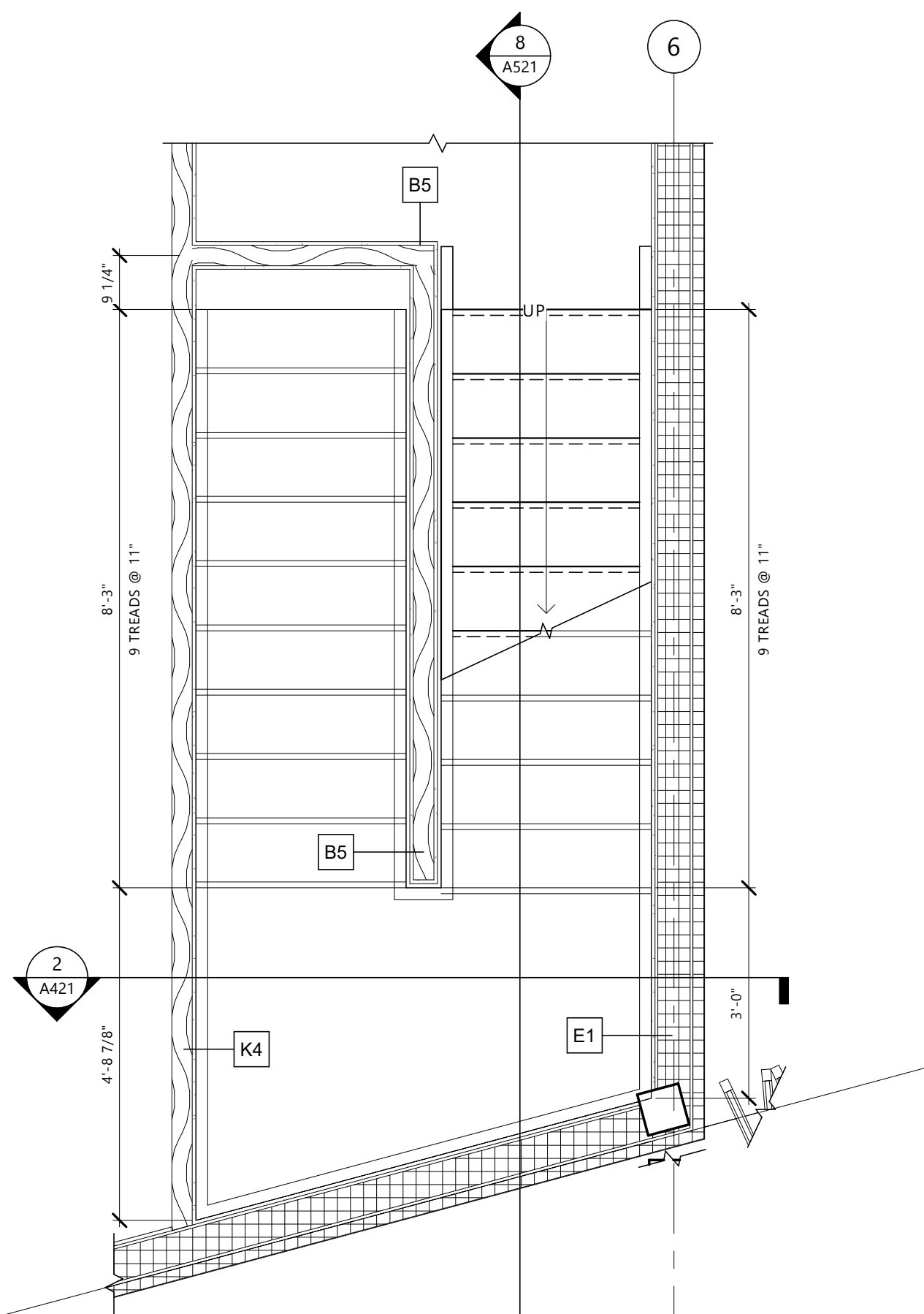
9 SOFFIT DETAIL
A521 1 1/2" = 1'-0"



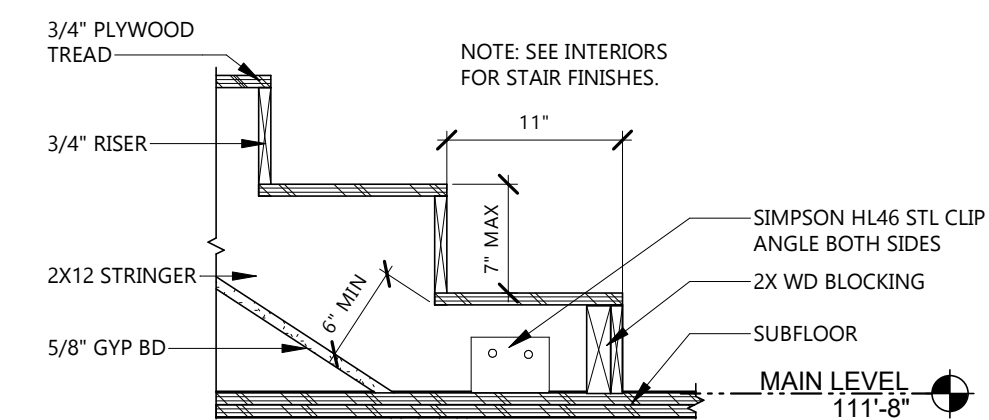
8 STAIR SECTION
A521 1/2" = 1'-0"



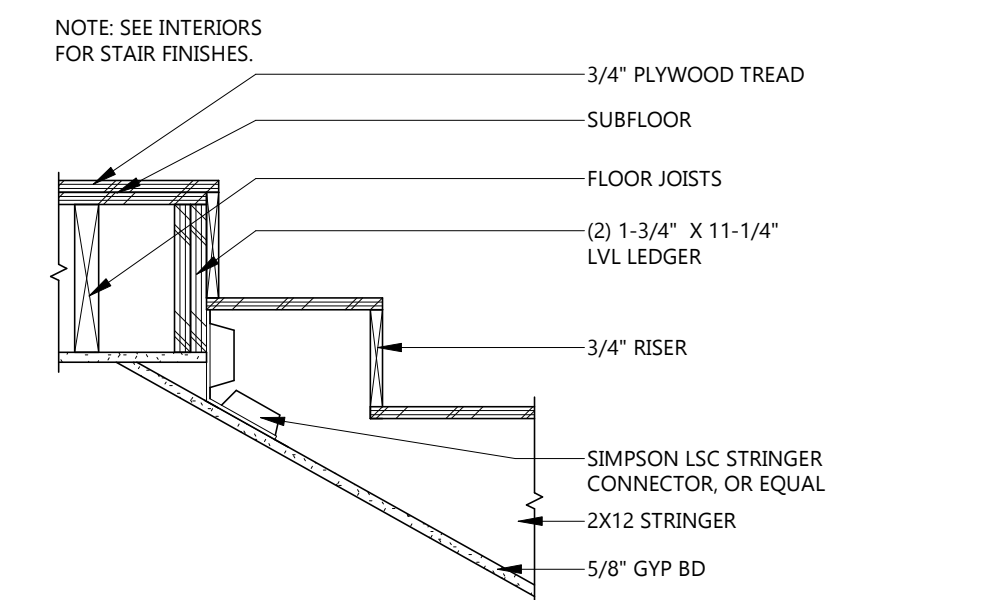
7 PRESS BOX STAIR PLAN
A521 1/2" = 1'-0"



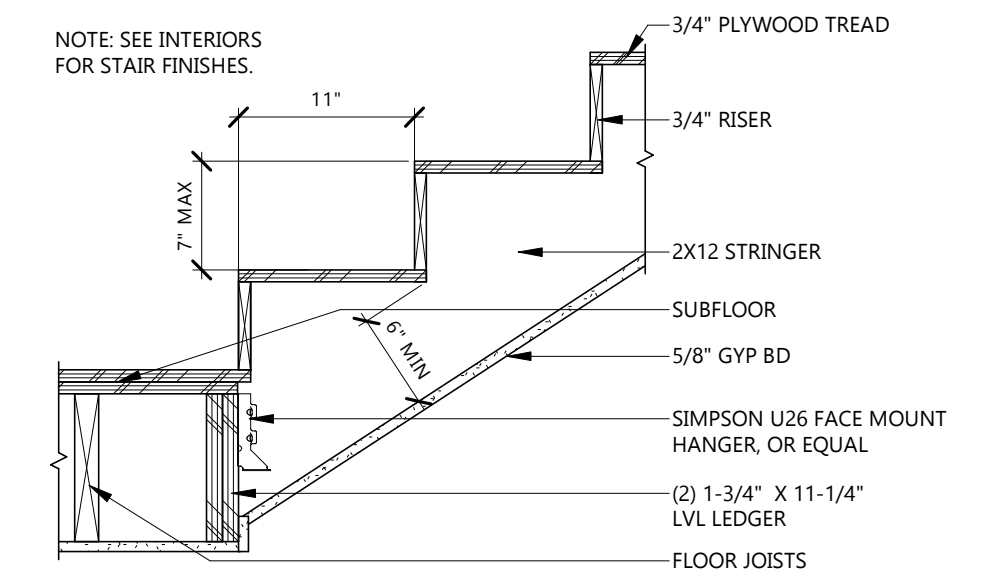
6 MAIN LEVEL STAIR PLAN
A521 1/2" = 1'-0"



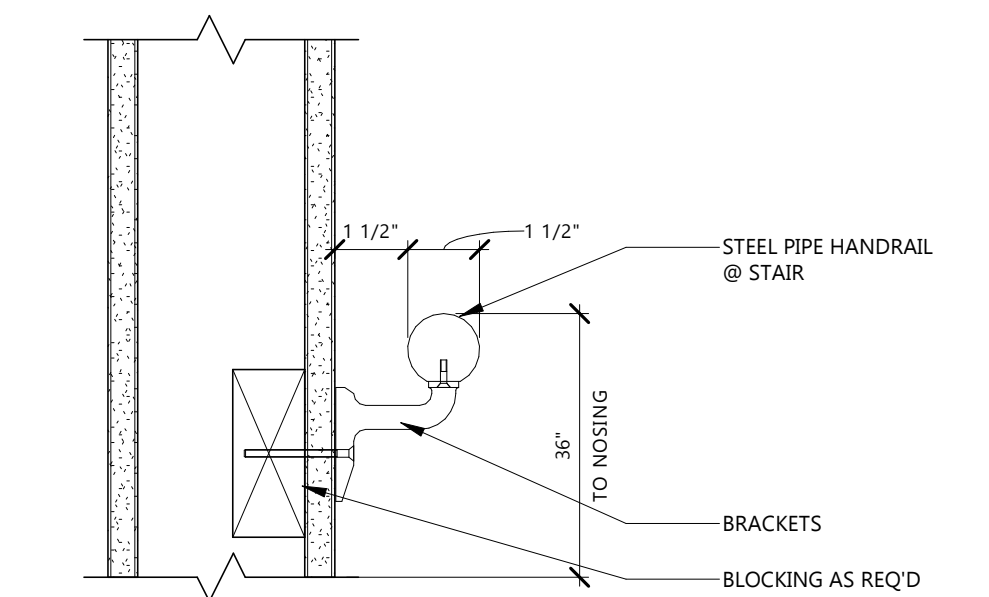
5 TYP. STAIR FOOT DETAIL
A521 1" = 1'-0"



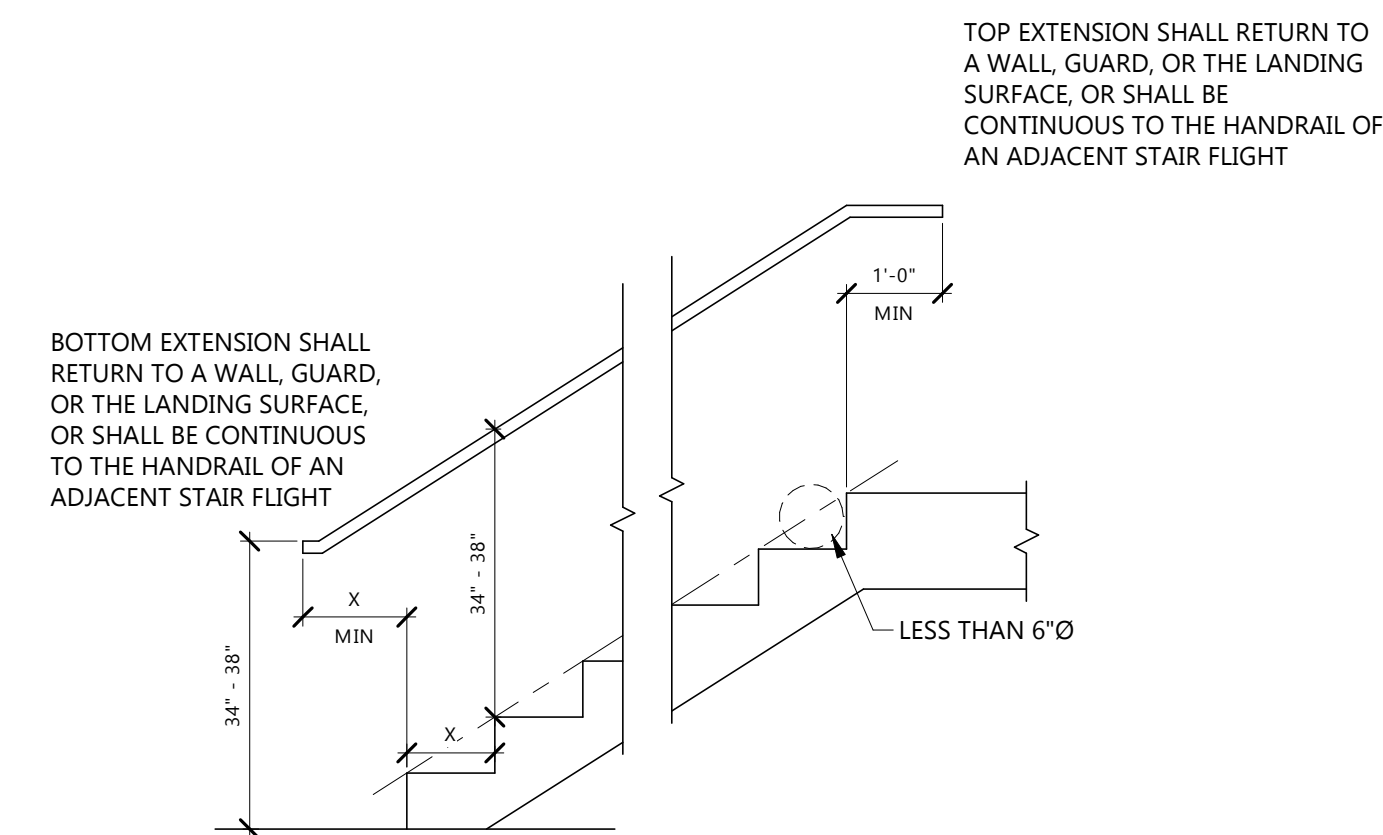
4 TYP. STAIR HEAD DETAIL
A521 1" = 1'-0"



3 TYP. STAIR FOOT DETAIL @ LANDING
A521 1" = 1'-0"



2 HANDRAIL DETAIL
A521 3" = 1'-0"



1 STAIR RAILING REQUIREMENTS
A521 1/2" = 1'-0"

ROOM FINISH SCHEDULE									
ROOM		FLOOR	BASE	WALLS				CEILING	NOTES
NO.	NAME			NORTH	EAST	SOUTH	WEST	FINISH	
101	FAMILY RESTROOM	T-1	T-1	P-1/ T-2	P-1/ T-2	P-1/ T-2	P-1/ T-2	WD-1	
102	FAMILY RESTROOM	T-1	T-1	P-1/ T-2	P-1/ T-2	P-1/ T-2	P-1/ T-2	WD-1	
103	WOMEN'S	T-1	T-1	P-1/ T-2	P-1/ T-2	P-1/ T-2	P-1/ T-2	WD-1	
104	STAIR	STR-1	RB-1	P-1	P-1	P-1	P-1	WD-1	
105	TICKETING	RTF-1	RB-1	P-1	P-1	P-1	P-1	WD-1	
106	JANITOR	RSF-1	RSF-1	P-1/ FRP-1	P-1/ FRP-1	P-1/ FRP-1	P-1/ FRP-1	WD-1	
107	MEN'S	T-1	T-1	P-1/ T-2	P-1/ T-2	P-1/ T-2	P-1/ T-2	WD-1	
108	CONCESSIONS	RSF-1	RSF-1	P-1/ FRP-2	P-1/ FRP-2	P-1/ FRP-2	P-1/ FRP-2	WD-1	

FINISH LEGEND	
PNT-X	WALL FINISH SYMBOL
CTF-X	FLOOR FINISH SYMBOL
EXG	EXISTING FLOOR FINISH TO REMAIN

- GENERAL FINISH NOTES
1.

REFER TO SHEET A001 FOR ADDITION GENERAL NOTES.
2.

REFER TO FINISH PLANS, INTERIOR ELEVATIONS, AND REFLECTED CEILING PLANS FOR ADDITIONAL FINISH LOCATIONS.
3.

REFER TO FINISH ELEVATIONS FOR PAINT COLOR, ARTWORK, AND GRAPHIC LOCATIONS.
4.

ALL AREAS OF CONCRETE SLAB RECEIVING A FLOOR COVERING SHALL UNDERGO MOISTURE TESTING IN ACCORDANCE WITH SPECIFICATIONS AND MANUFACTURERS INSTALLATION. SUBMIT CONCRETE MOISTURE TESTING REPORT WITH DATA SHOWING COMPLIANCE.
5.

CONTRACTOR SHALL INSTALL ALL MATERIALS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND SPECIFICATIONS, INCLUDING REQUIREMENTS AFFECTING PRODUCT WARRANTIES.
6.

ALL TRANSITIONS OF FLOOR FINISH MATERIALS SHALL BE LOCATED DIRECTLY UNDER CENTER OF DOOR, WHERE OCCURS, UNO, AT SLIDING DOORS, TRANSITIONS SHALL BE LOCATED ON THE INSIDE FACE OF DOOR, UNO.
7.

ALIGN TILE TRANSITIONS ON WHOLE TILE WHERE POSSIBLE.
8.

TILE BASE AND THRESHOLD GROUT JOINTS TO ALIGN WITH TILE FLOOR GROUT JOINTS.
9.

PROVIDE GROUT JOINT OF 3/16" (5MM). MAXIMUM DEVIATION OF TILE AND GROUT LINE OF 1/4" PER 48"
10.

ALL WOOD SURFACES (DOOR, TRIM, SHELVES, CABINETRY, ETC) SHALL BE SEALED.
11.

ALL SEALANTS, EXPOSED TO VIEW, ARE TO MATCH SURFACE IN WHICH THEY OCCUR. FINAL COLOR TO BE SELECTED BY ARCHITECT.
12.

JOINTS IN 1/8" >RESILIENT BASE SHALL BE NO LESS THAN 6" FROM CORNER AND SEALED TIGHTLY TOGETHER. CORNERS NEED TO BE JOB FORMED ONSITE, UNO.
13.

REMOVE ALL PAINT FROM WHERE IT HAS SPILLED, SPLASHED, OR SPLATTERED ON EXPOSED SURFACES.
14.

FOR STAINED AND PAINTED SURFACES, ALL JOINTS SHALL BE FINISHED AND NAIL HOLES FILLED TO PRODUCE A CONSISTENT APPEARANCE.
15.

PREPARE ALL CORNERS SHOWN ON A901 AND A903 WHERE SPECIALTY CORNER GUARD INDICATED.
16.

PROVIDE CORNER GUARDS AT OUTSIDE CORNERS, IN ALL PUBLIC SPACE OR AS INDICATED ON FINISH PLAN.
17.

FULL HEIGHT CORNER GUARDS TYPICAL AT ALL PUBLIC AND MULTI-OCCUPANT SPACES AND WHERE MATERIAL TYPE/ COLOR/ FINISH CHANGES.
18.

CONTRACTOR TO SUBMIT FLOOR SEAMING DIAGRAM APPROVAL PRIOR TO FABRICATION AND INSTALLATION.
19.

EXPOSED TO VIEW STRUCTURE, TO RECEIVE DRYWALL PAINT, UNO. COLOR AS SELECTED.
20.

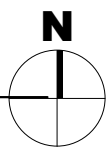
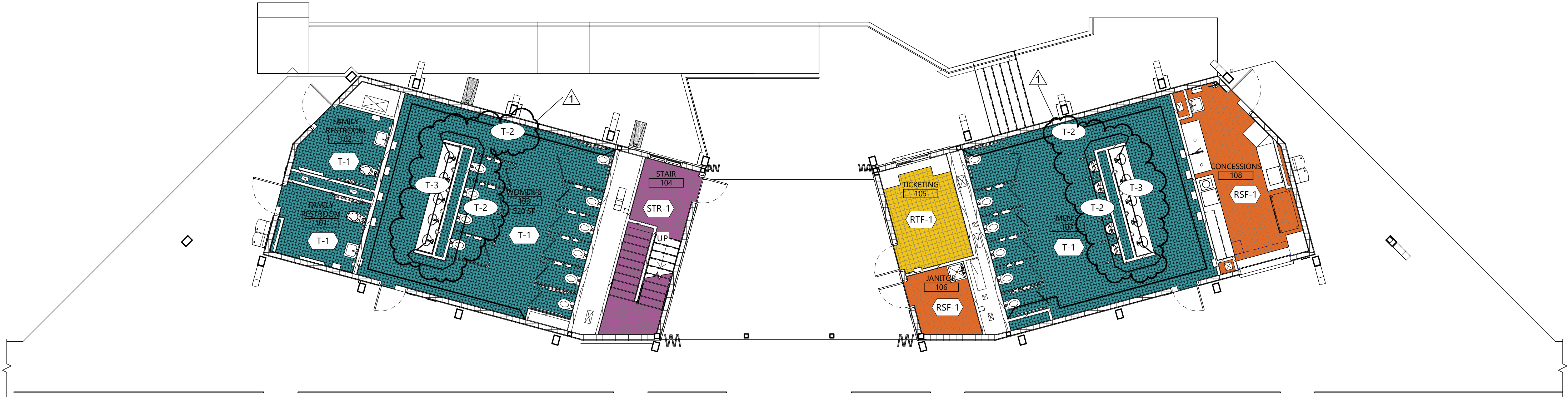
CHANGE OF PAINT COLOR AT INSIDE CORNERS, UNO.
21.

REMOVE SURFACE IMPERFECTIONS AND PAINT ALL EXPOSED TO VIEW STEEL COLUMNS.
22.

ALL TILE INSTALLATIONS SHALL INCLUDE MOVEMENT JOINTS COMPLIANT WITH THE CONDITIONS, LOCATIONS, AND METHODS DESCRIBED BY THE MOST RECENTLY PUBLISHED VERSION OF THE TILE COUNCIL OF NORTH AMERICA (TCNA) HANDBOOK FOR CERAMIC, GLASS, AND STONE INSTALLATION MOVEMENT JOINT GUIDELINES. MOVEMENT JOINTS SHALL BE FIELD COORDINATED WITH ARCHITECT PRIOR TO INSTALLATIONS.
23.

SEE STRUCTURAL FOR CONCRETE SUBFLOOR JOINT LOCATIONS INTERIOR FINISHES ARE TO BE COORDINATED WITH STRUCTURAL EXPANSION JOINTS. GC TO SUBMIT CONTROL JOINT PLAN FOR APPROVAL. SEE DETAILS.

KEYNOTE LEGEND:	
< < <	INDICATES KEYNOTE ON PLAN



EAPC

Architecture

Engineering

Interior Design

Industrial

TELEPHONE701.258.3116

116 W. Main Ave., Suite A, Bismarck ND 58501

www.eapc.net

CONSULTANTS

CLIENT
NEW TOWN SCHOOL DISTRICT

PROJECT DESCRIPTION
NEW TOWN GRANDSTANDS

CITYNEW TOWN
STATEND

ISSUE DATES

1	ADDENDUM 01	08/26/2026
CD	CONSTRUCTION DOCUMENTS	08/17/2026
MARK	DESCRIPTION	DATE

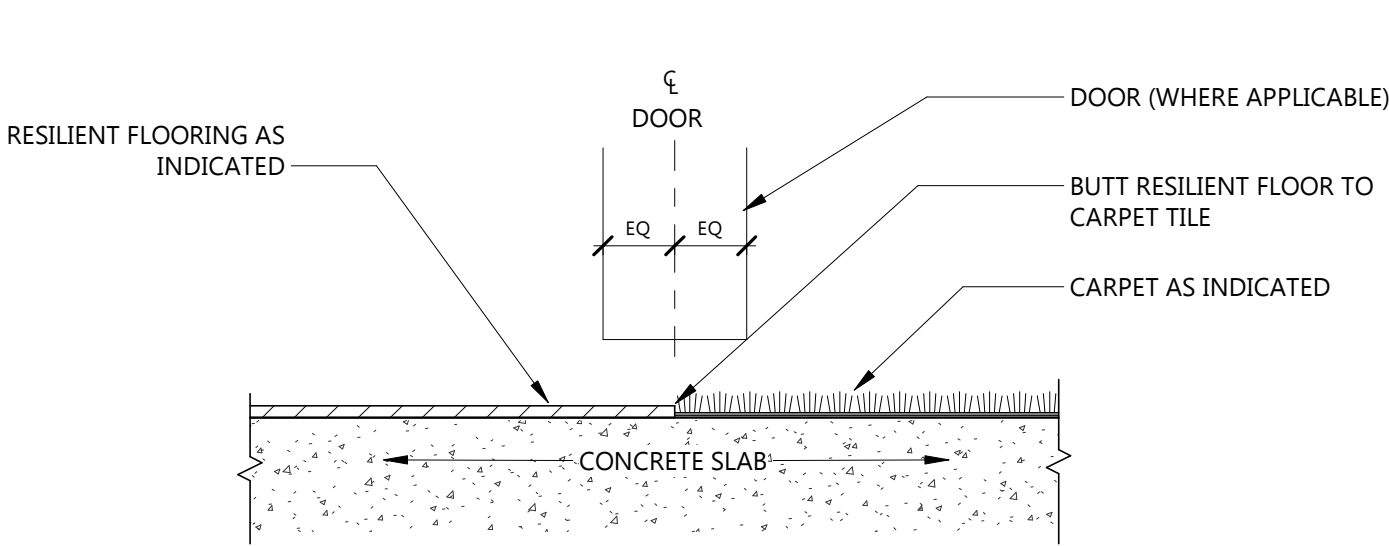
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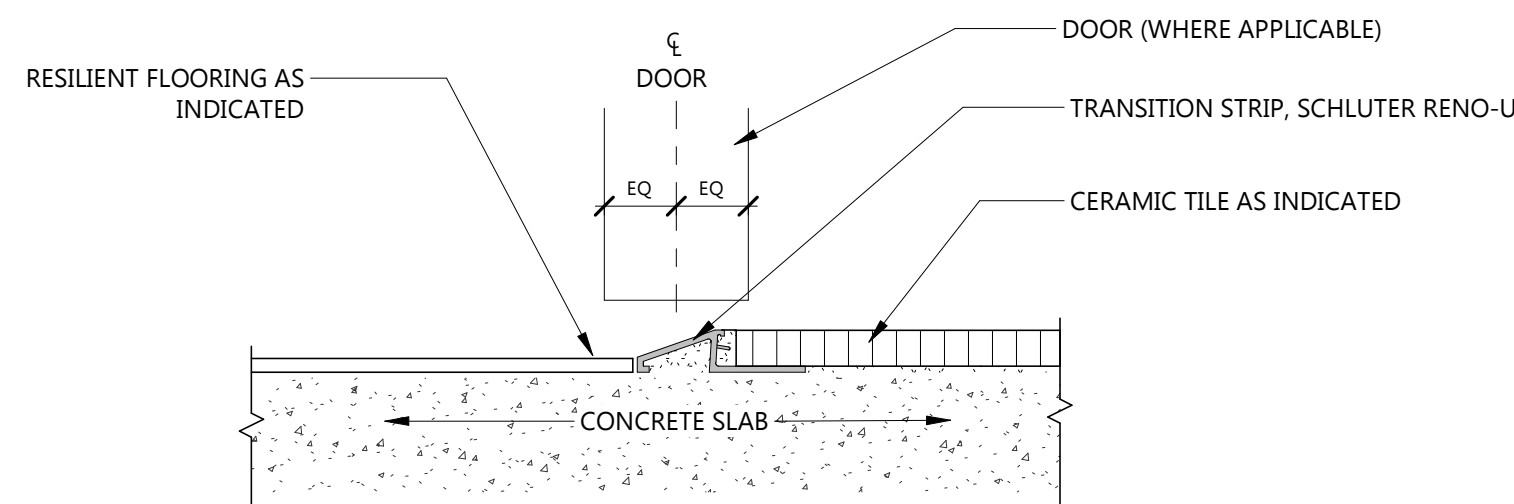
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FINISH PLAN

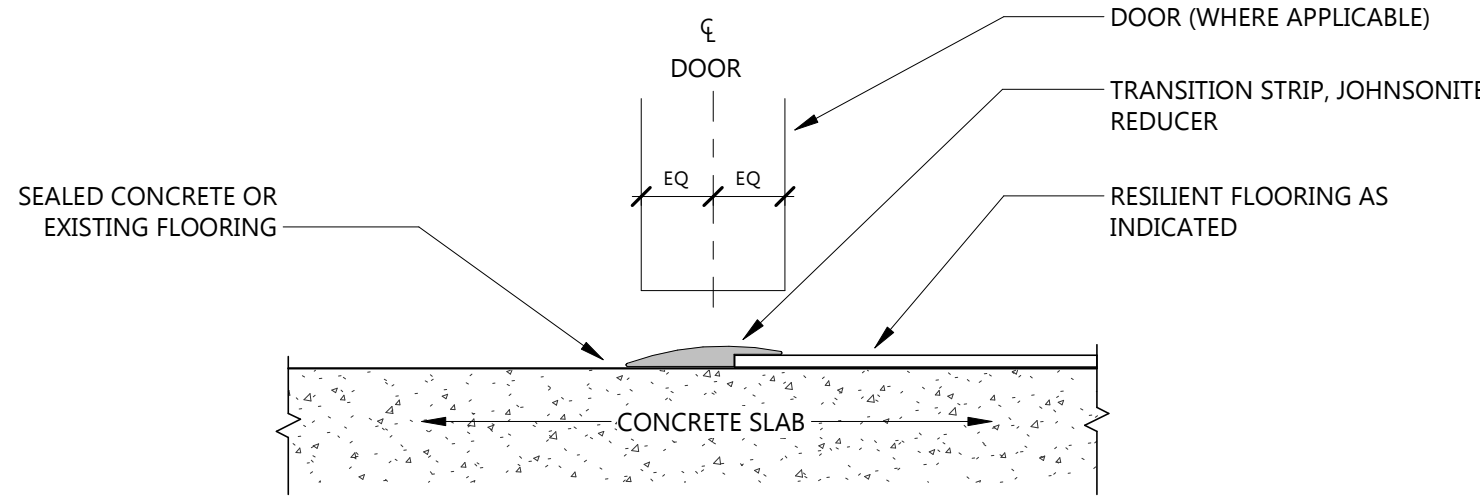
A902



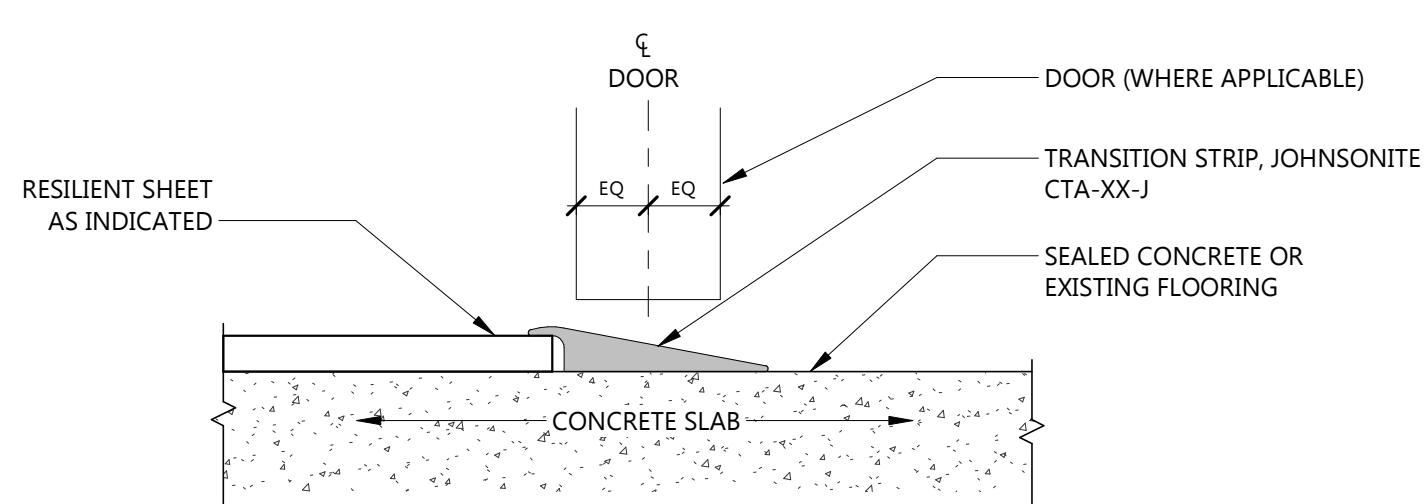
6 TS-4 RESILIENT FLOORING TO CARPET
A921 NOT TO SCALE



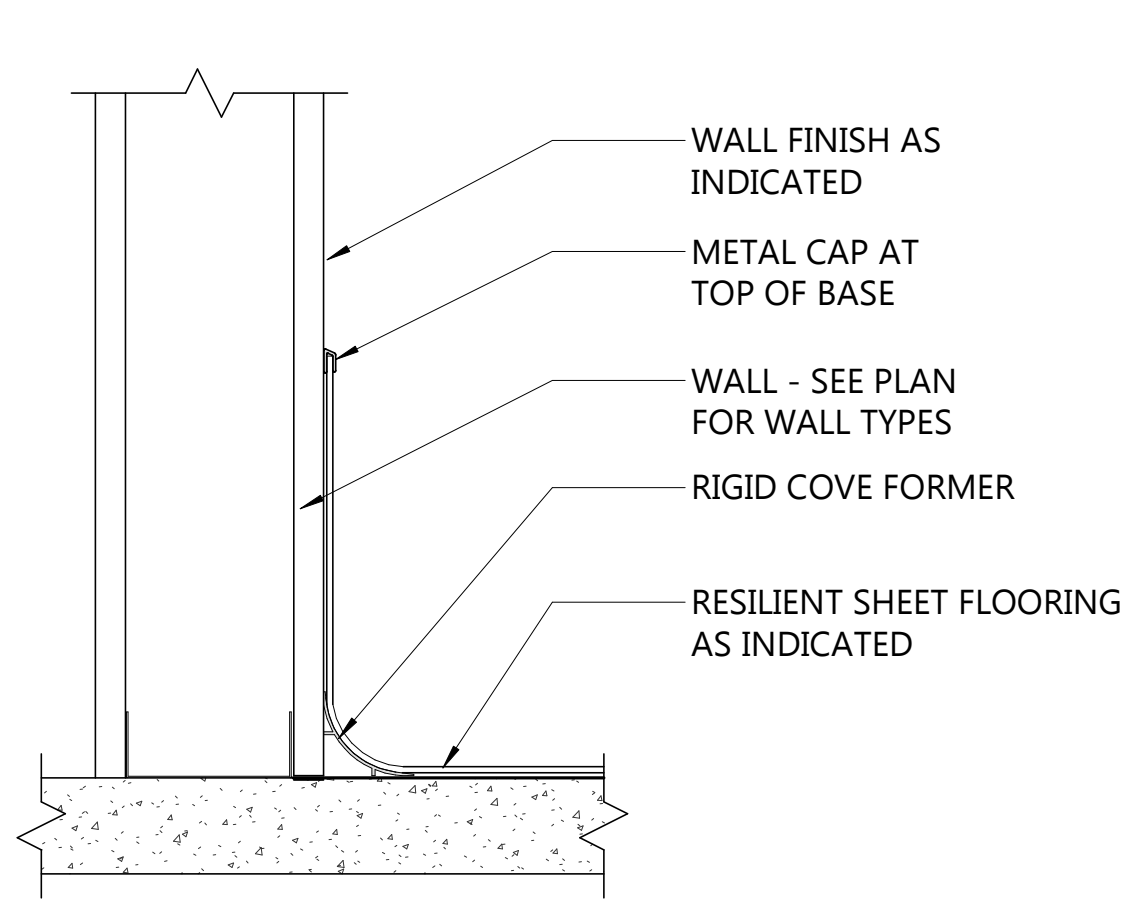
5 TS-2 RESILIENT FLOORING TO CERAMIC TILE
A921 NOT TO SCALE



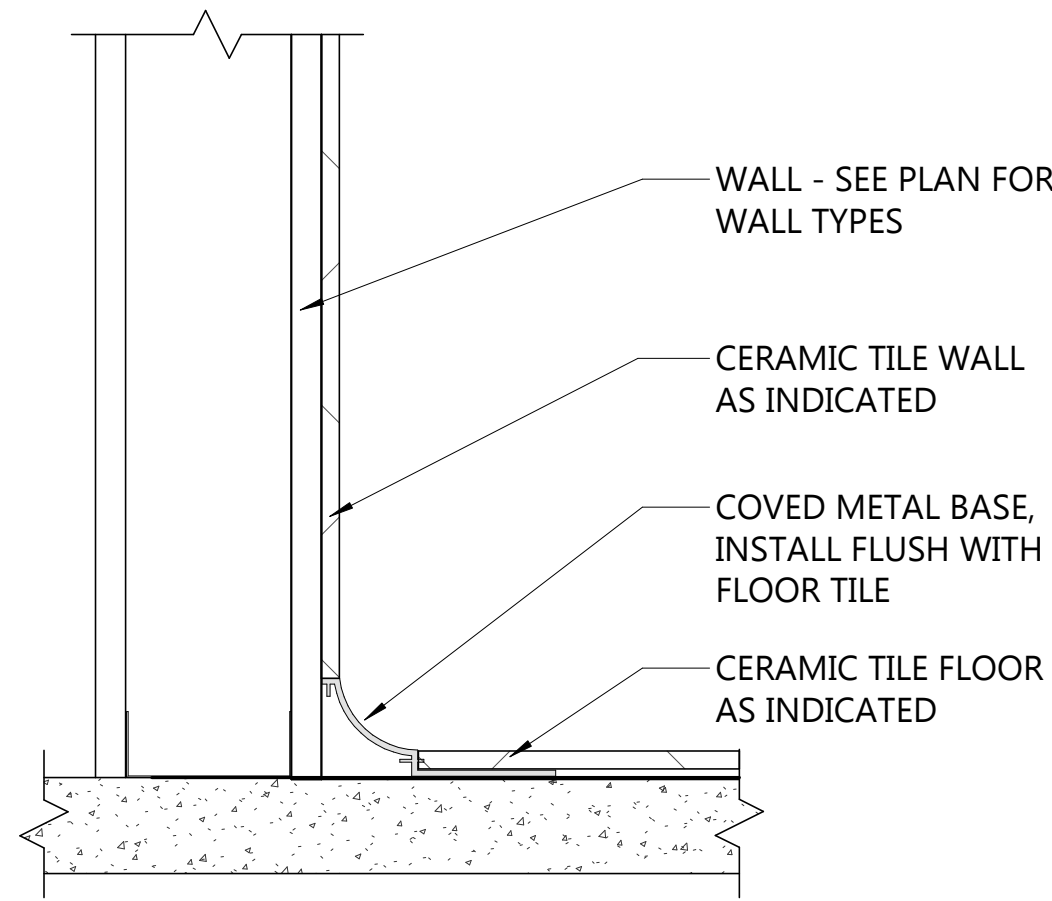
4 TS-3 SEALED CONCRETE TO RESILIENT FLOORING
A921 NOT TO SCALE



3 TS-1 RESILIENT SHEET TO SEALED CONCRETE
A921 NOT TO SCALE



2 INTEGRAL COVE BASE
A921 NOT TO SCALE



1 CERAMIC WALL TILE TO CERAMIC FLOOR TILE
A921 NOT TO SCALE

FINISH LEGEND						
SPEC. DIV.	FINISH CODE	MANUFACTURER	PRODUCT DESCRIPTION	COLOR/ SKU	INSTALLATION	NOTES
CONCRETE FLOOR FINISHES						
03 3511	CFS-1	SHERWIN WILLIAMS,	H&C CLARISHIELED WATER-BASED WET-LOOK CONCRETE SEALTER			
ARCHITECTURAL WOOD CASEWORK						
06 4100	PL-1	WILSONART	PREMIUM LAMINATE	ASIAN SUN 7951K-18		
06 4100	PL-2	WILSONART	VIRTUAL DESIGN LIBRARY	IOLITE Y0737-60		
FIBERGLASS REINFORCED PANELING						
06 8316	FRP-1	MARLITE	STANDARD FRP, PEBBLE FINISH	LIGHT GRAY P-151		
06 8316	FRP-2	MARLITE	SYMMETRIX FRP, SUBWAY 3"X6" HORIZONTAL CONFIGURATION	LOGGIA		
TILING						
09 3000	T-1	CERAMIC TILEWORKS MN	RETINA, 12"X24"	BORGOGNA	STRAIGHT STACK	GROUT: LATICRETE, SPECTRALCOK PRO PREMIUM GROUT
09 3000	T-2	CERAMIC TILEWORKS MN	RETINA, 12"X24"	NEVE	STRAIGHT STACK	GROUT: LATICRETE, SPECTRALCOK PRO PREMIUM GROUT
09 3000	T-3	CERAMIC TILEWORKS MN	RETINA, 1"X6" MOSAIC SHEET 12"X12"	BORGOGNA	STRAIGHT STACK	GROUT: LATICRETE, SPECTRALCOK PRO PREMIUM GROUT
09 3000	TS-2	SCHLUTER	RSF-2 TO T-1			
LINEAR METAL CEILINGS						
09 5423	WD-1	LONGBOARD	LONGBOARD INSPIRING ARCHITECTURE, ENDURA LINEAR CEILINGS			
RESILIENT BASE AND ACCESSORIES						
09 6513	RB-1	JOHNSONITE	BASEWORKS THERMOSET RUBBER (TYPE TS) 4" COVE			
09 6513	STN-1	JOHNSONITE	STAIR NOSING, RCN-XX-A			
09 6513	STR-1	NORA BY INTERFACE	NORAMENT 926 GRANO STAIRTREAD	BLACK PEPPER 5304		
09 6513	TS-1	JOHNSONITE	CFS TO RSF-2			
09 6513	TS-3	JOHNSONITE	RTF TO CFS			
09 6513	TS-4		RTF TO CPT			
RESILIENT SHEET FLOOR						
09 6516	RSF-1	TARKETT	ARIA, HOMOGENEOUS SHEET	DARK CLAY GRAY CB 629		
RESILIENT TILE FLOOR						
09 6519	RTF-1	J&J FLOORING GROUP	PASSPORT V5049, 18"X36"	CONVENIENT 1148	ASHLAR	
RESILIENT ATHLETIC FLOORING						
09 6566	RSF-2	TARKETT	TRIUMPH MINERAL SPORTS FLOORING, SMHMR-XX, 42399200460	JAY WQ4		
TILE CARPETING						
09 6813	CPT-1	J&J FLOORING GROUP	DOWNTOWN 1850, 24"X24"	MARKET 3157	ASHLAR	75% MARKET, 25% SUNSET
09 6813	CPT-2	J&J FLOORING GROUP	DOWNTOWN 1850, 24"X24"	SUNSET 3158	ASHLAR	75% MARKET, 25% SUNSET
SOUND-ABSORBING WALL AND CEILING UNITS						
09 8430	AWP-1	ZINTRA	BOX TILE EMERALD	TBD		
PAINTING AND COATING						
09 9000	EPX-1					
09 9000	P-1	GLIDDEN	EGG SHELL	DESERT SAND	FIELD PAINT	TO MATCH OWNERS BUILDING STANDARD
09 9000	P-2	SHERWIN WILLIAMS	FLAT	HIGH REFLECTIVE WHITE SW 7757	CEILING PAINT	
PLASTIC TOILET COMPARTMENTS						
10 2113.19	PTC-1	SCRANTON PRODUCTS	HINY HIDERS PARTITIONS	STAINLESS GRIP EX		
WALL PROTECTION - CORNER GUARDS						
10 2600	CG-1	INPRO	SURFACE MOUNT STAINLESS STEEL CORNER GUARD 2" WING	STAINLESS STEEL		
METAL LOCKERS						
10 5113	BEN-1	SCRANTON PRODUCTS	TUFFTEC LOCKER BENCHES			
10 5113	ML-1	REPUBLIC STORAGE PRODCUTS	METAL LOCKERS	WOOD VIOLET 54		
10 5113	ML-2	REPUBLIC STORAGE PRODCUTS	METAL LOCKERS	SUNSHINE 97		
HORIZONTAL LOUVER BLINDS						
12 2113	WDT-1	HUNTER DOUGLAS ARCHITECTURAL	RIVIERA COMMERCIAL HORIZONTAL BLINDS			
SOLID SURFACING COUNTERTOPS						
12 3661.16	SSF-1	DEKTON	ALBARIUM 22, KRAFTIZEN COLLECTION	QUICK CUT 2448/11102023		
QUARTZ COUNTERTOP						
12 3661.19	QZ-1	CAMBRIA USA	LUXURY SERIES	ANNICCA MATTE		

CONSULTANTS

CLIENT
NEW TOWN SCHOOL DISTRICT

PROJECT DESCRIPTION
NEW TOWN GRANDSTANDS

CITY **NEW TOWN**
STATE **ND**

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A921